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London 2019

Centrifuge modelling for geotechnical instruction & Visualisation modelling

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Research



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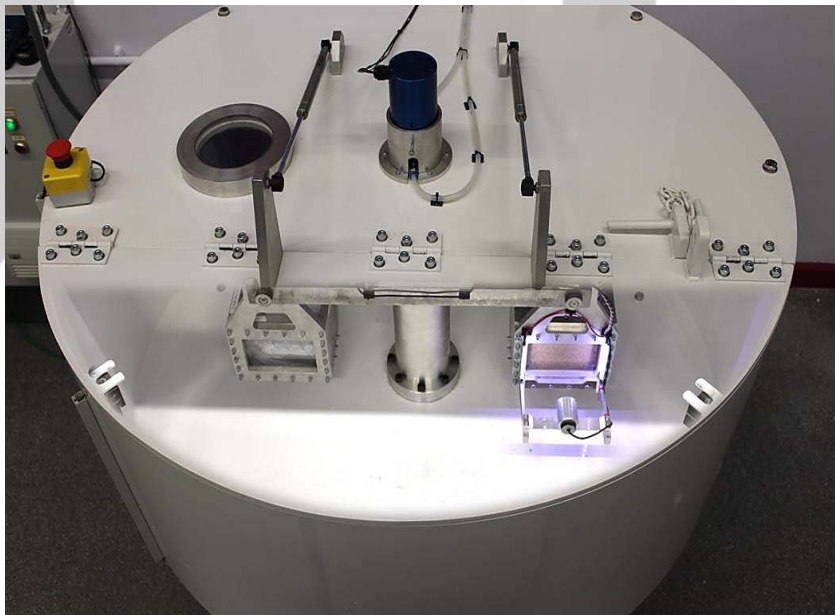




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- 2012 – 1m centrifuge commissioned
- 2014 – £1.2m EPSRC grant to for CEIGR
- 2015 – 4m centrifuge commissioned

1m diameter



4m diameter – 50gT





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CEIGR

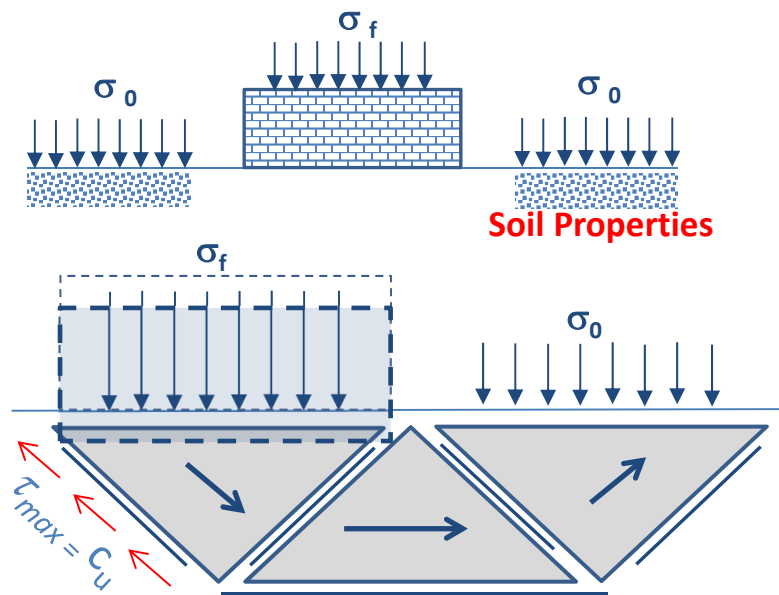






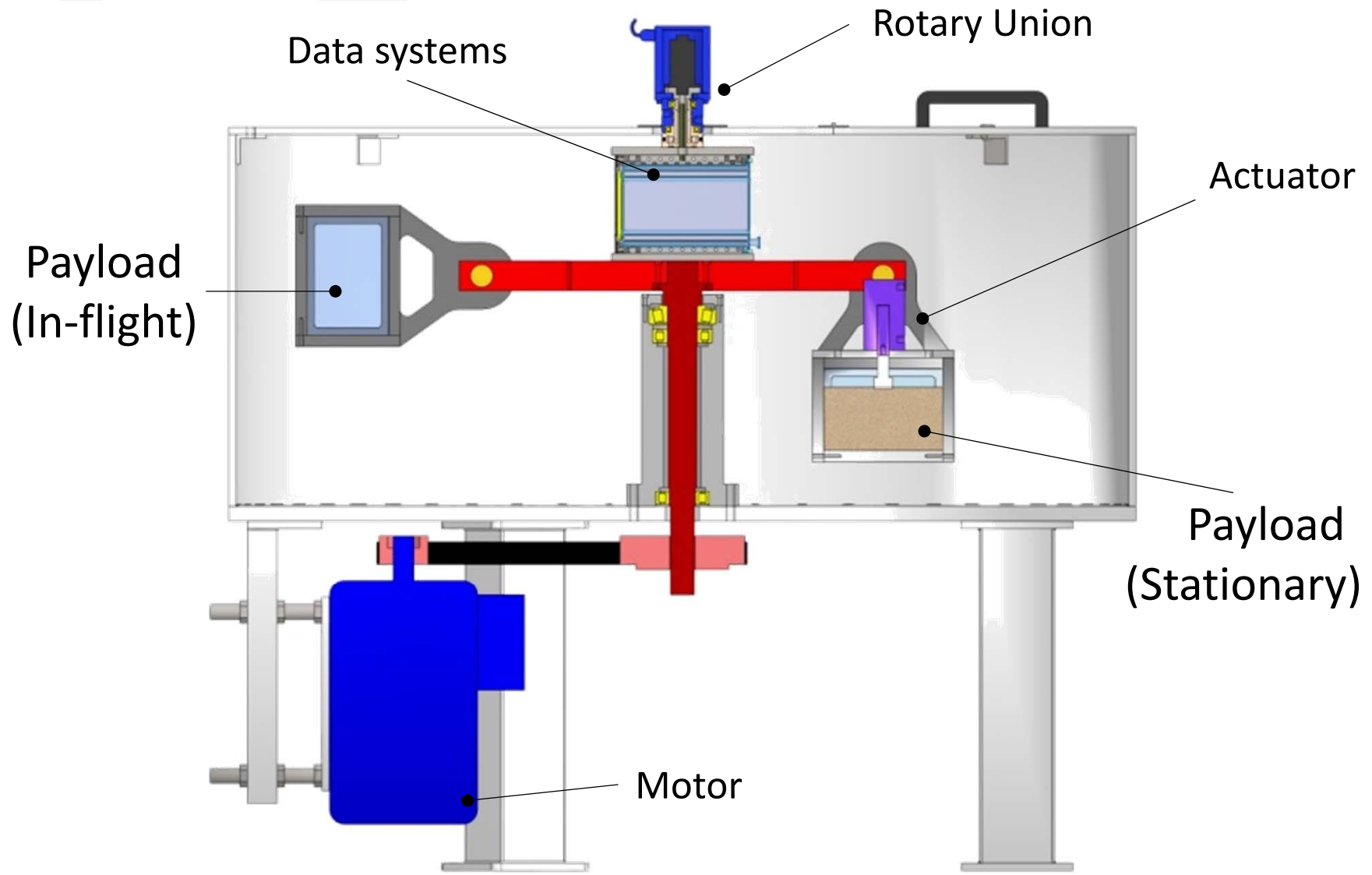
⦿ The laboratory is an essential learning environment

- Understanding of soil as an engineering material
- Reinforce & validate classical soil mechanics theory
- Develop laboratory skills - practical, data collection & appraisal



$$\sigma_f = c_u \cdot N_c + q_0$$







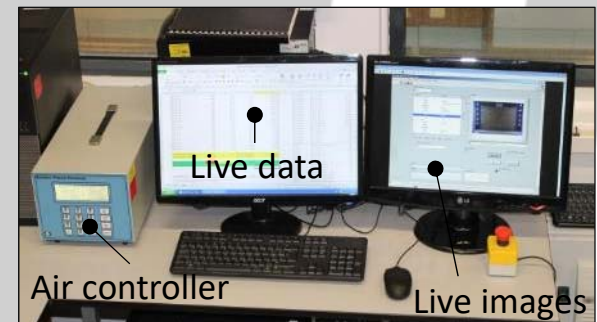
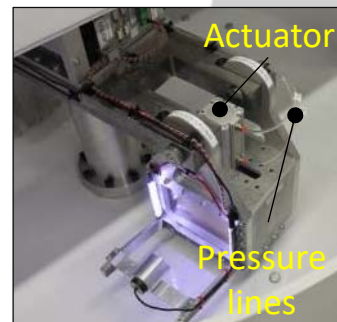
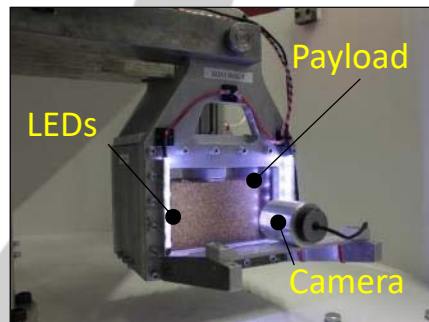
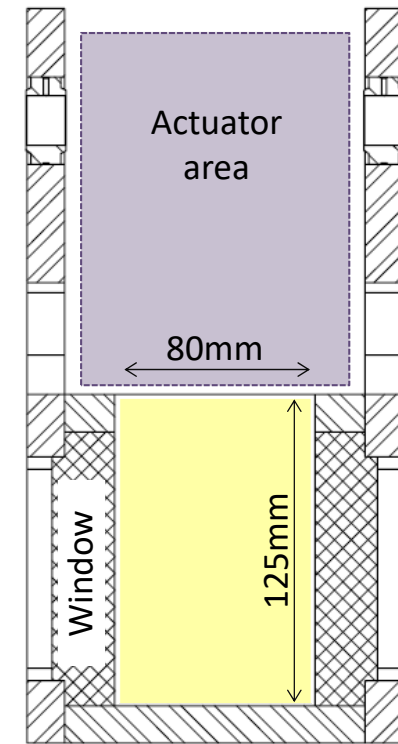
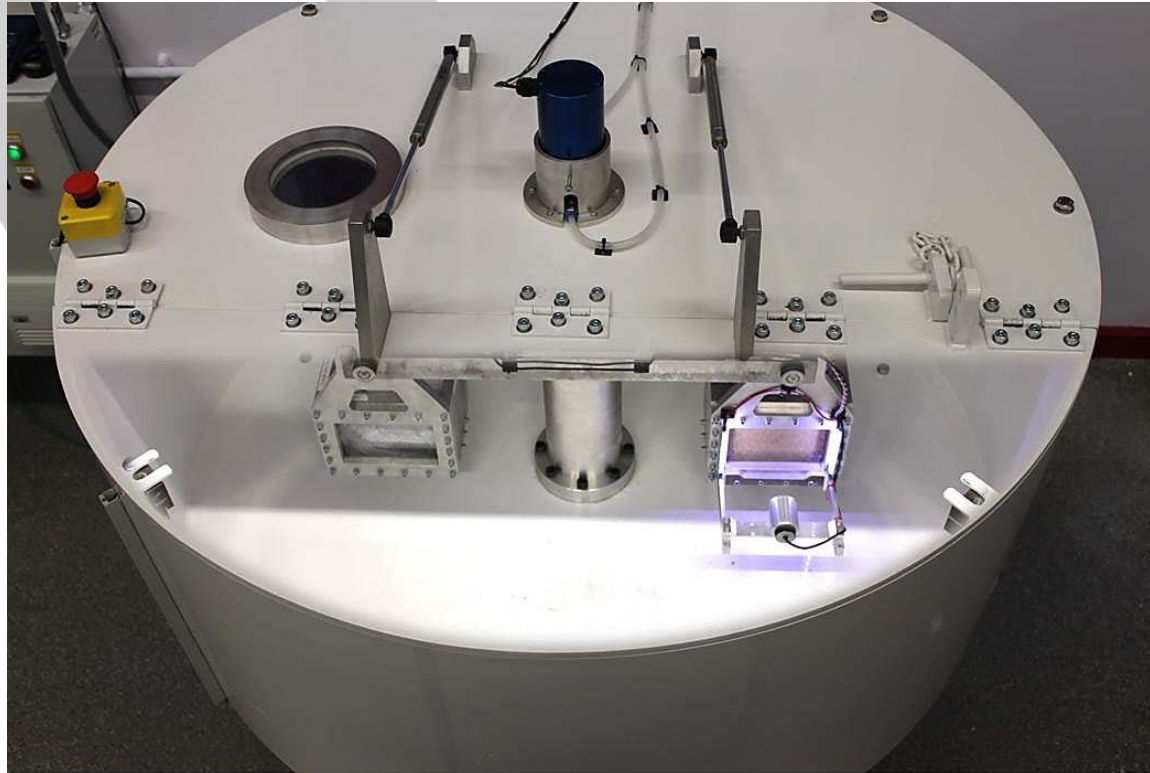
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Centrifuge modelling in the UG curriculum

UoS2gt/1.0 teaching centrifuge





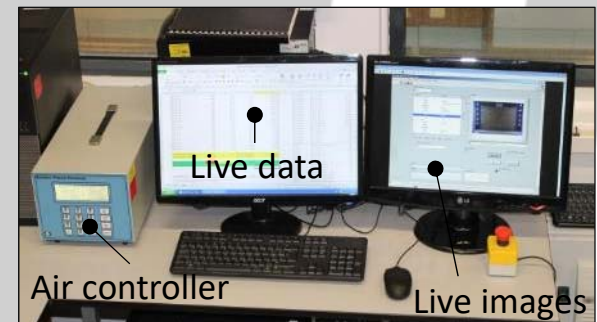
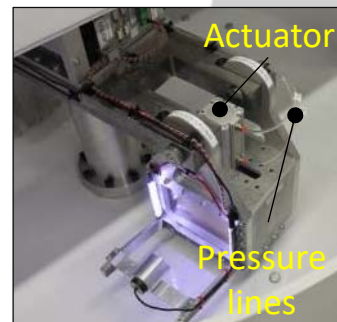
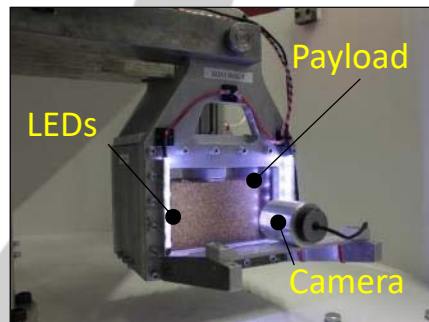
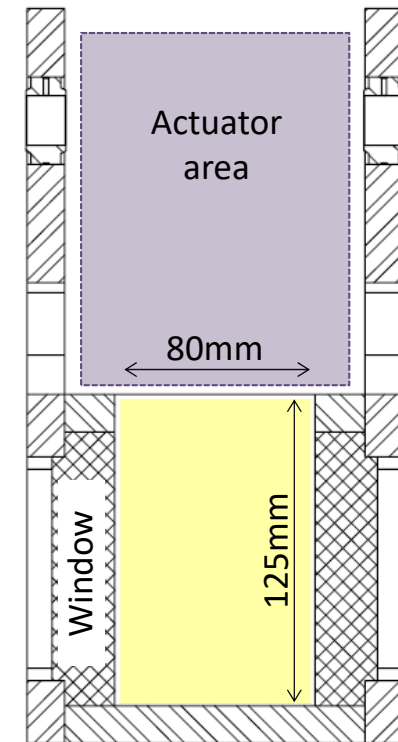
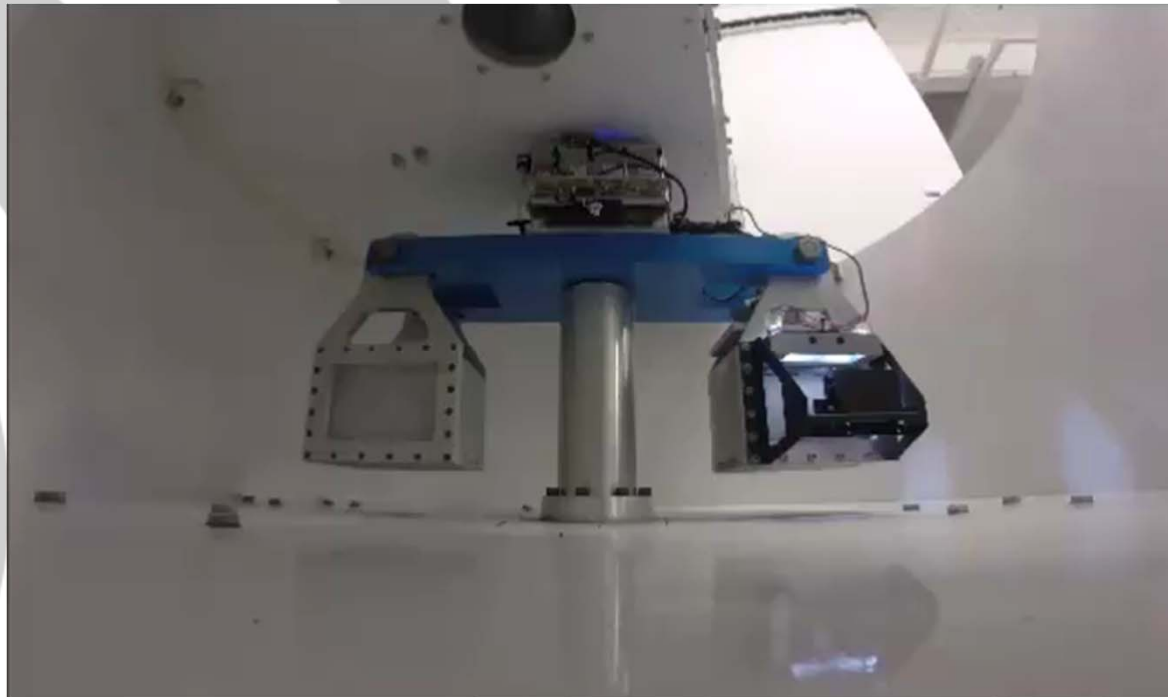
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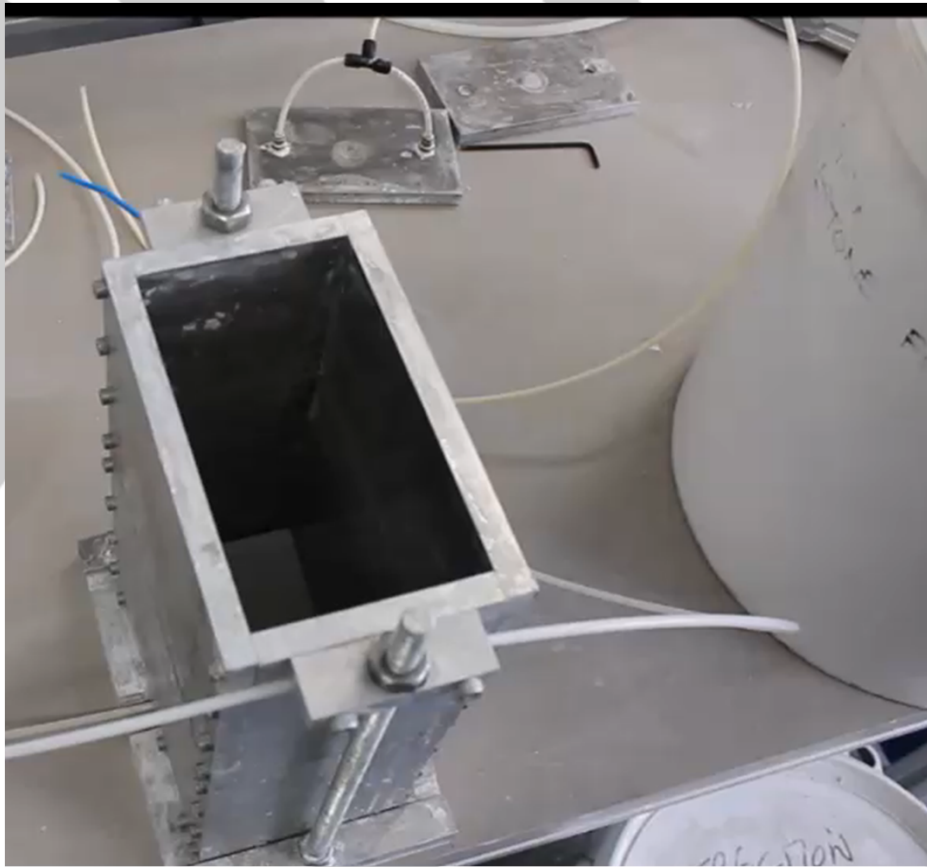
UoS2gt/1.0 teaching centrifuge





Model preparation

- Multiple samples is vital
- Repeatability important





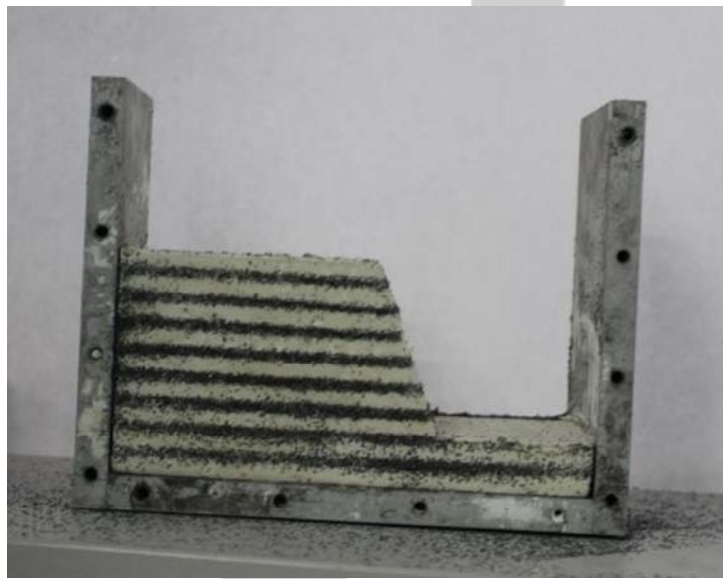
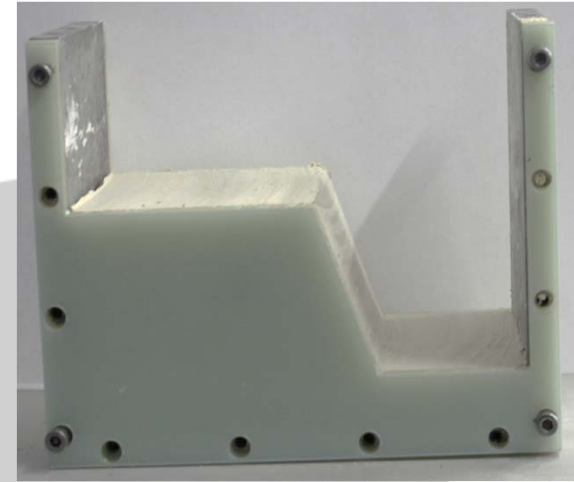
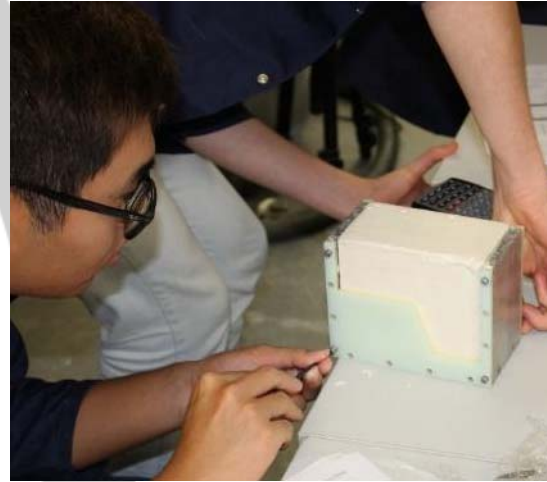
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Model preparation





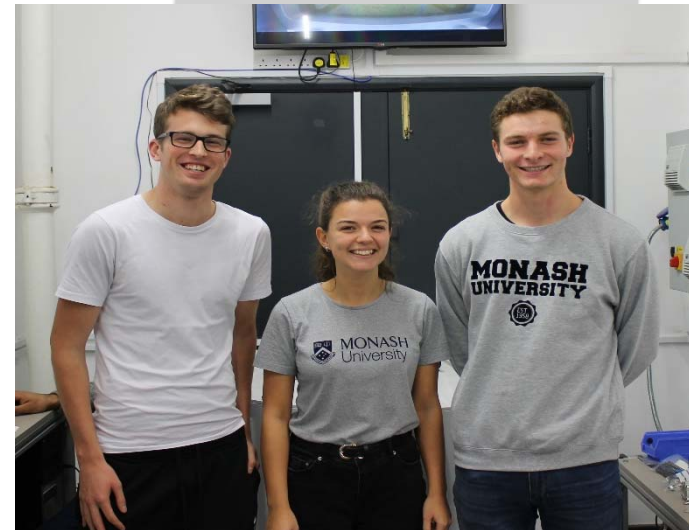
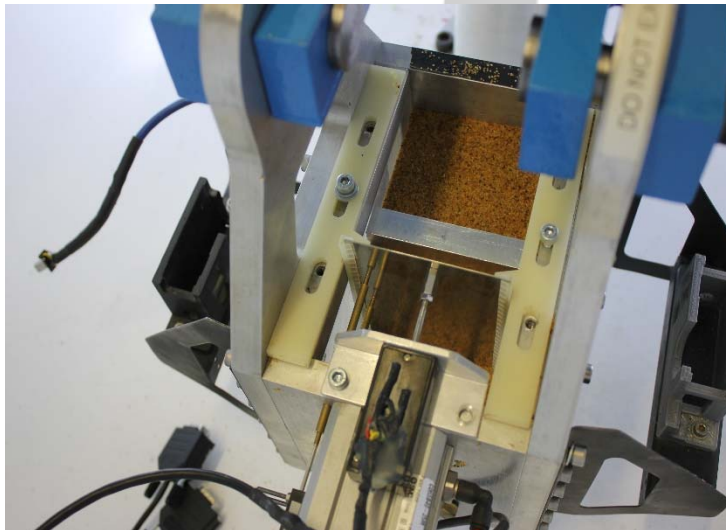
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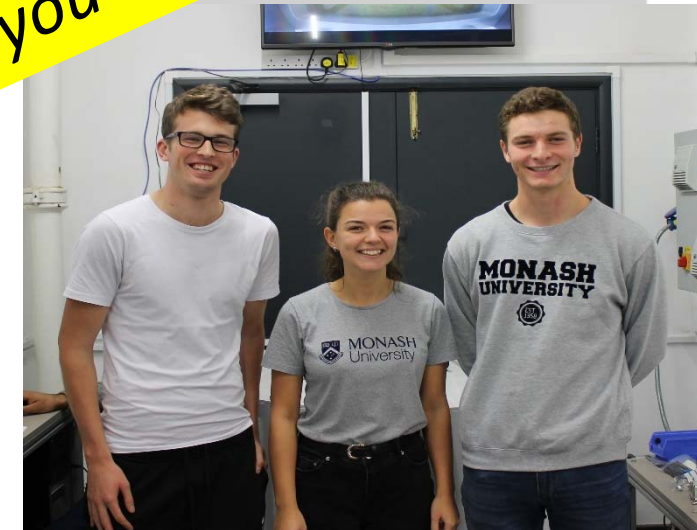
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Model preparation



Resist the temptation to interfere !!
'Are you really sure you want to do that?'





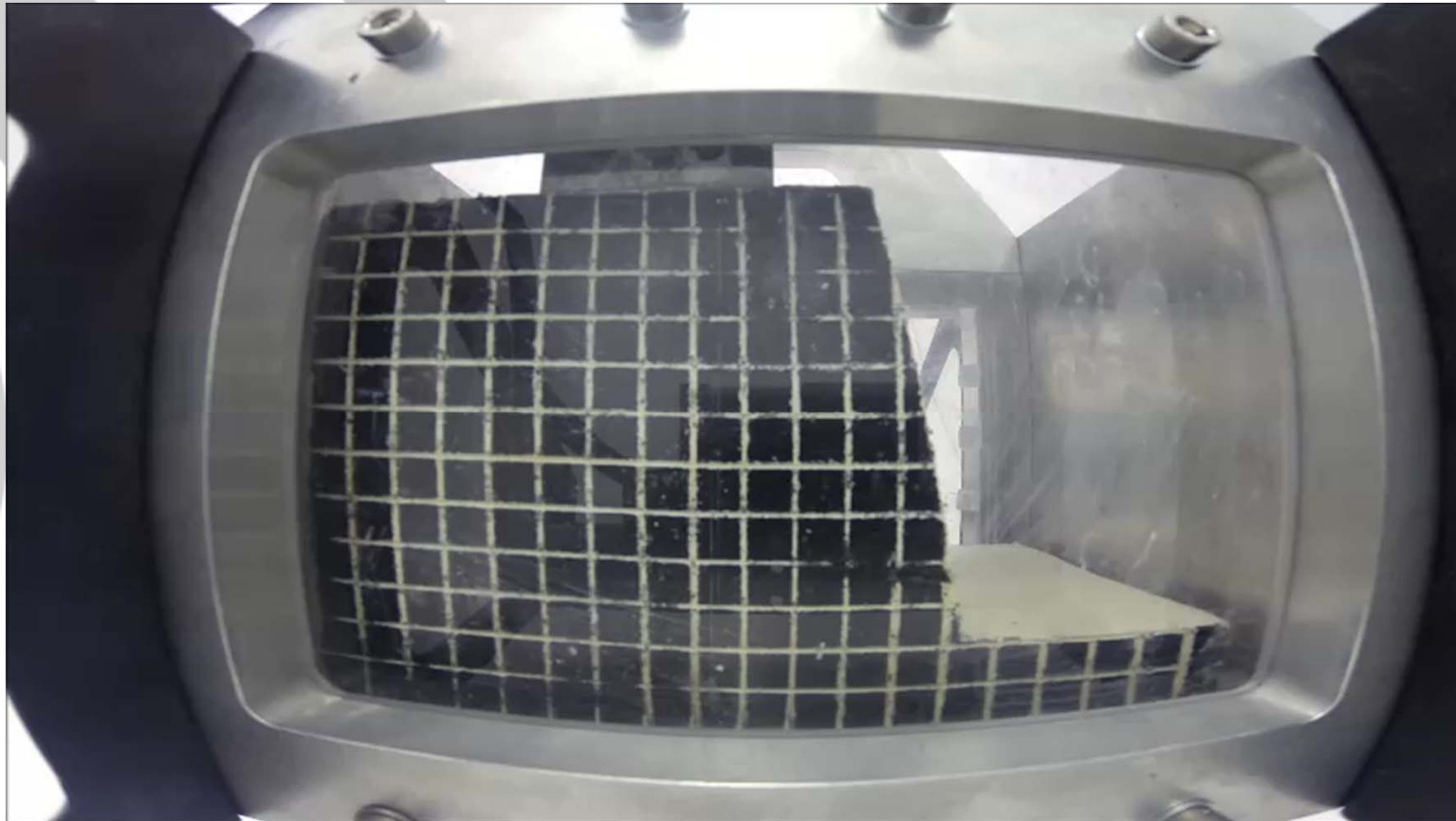
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Centrifuge modelling in the UG curriculum

Model preparation



Slopes

Footings

Retaining Walls

Flood embankments





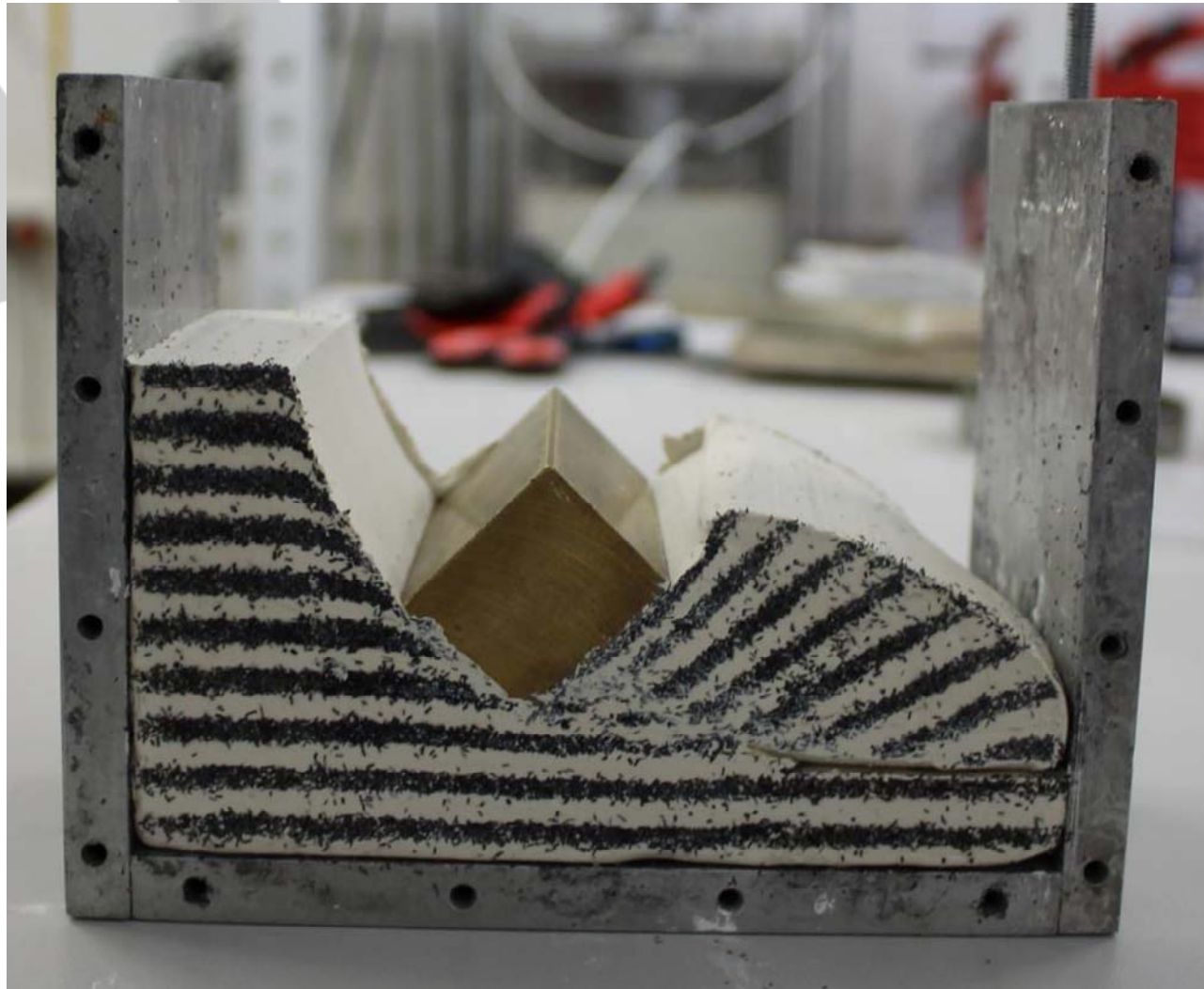
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Centrifuge modelling in the UG curriculum

Model preparation





Bespoke centrifuge module

- Unfamiliar problem – investigative approach
 - Analytical, numerical and experimental
- Suite of experimental results – element & centrifuge
 - Strength properties of soil
 - Observe behaviour of model
- Shared sense of responsibility to achieve high quality test data
- Individual student paper for conference dissemination





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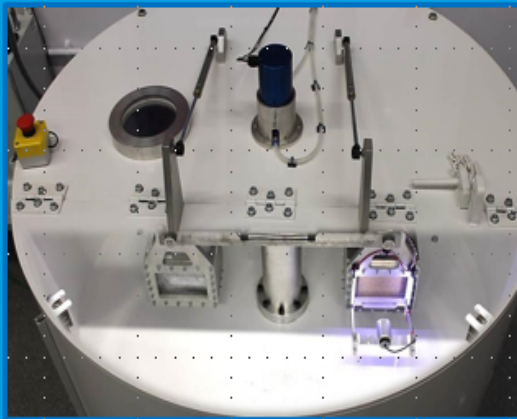


Centrifuge modelling in the UG curriculum

Design examples

Student Conference

PROCEEDINGS OF THE ADVANCED GEOTECHNICS
[CIV4501] STUDENT CONFERENCE ON CENTRIFUGE
MODELLING. SHEFFIELD, UK, 4TH NOVEMBER 2015.



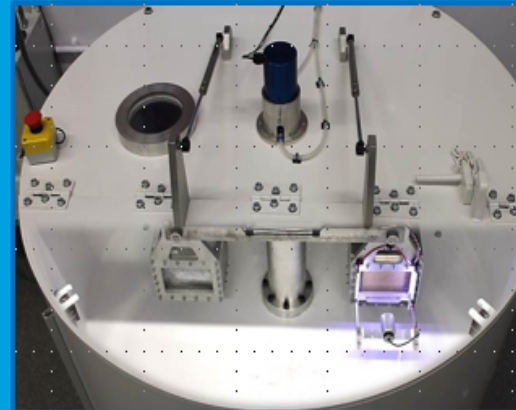
**ADVANCED GEOTECHNICS: CENTRIFUGE
MODELLING OF SLOPE STABILITY**

Editor: Jonathan A. Black

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**ADVANCED GEOTECHNICS: CENTRIFUGE
MODELLING OF RETAINING WALLS**

Editor: Jonathan A. Black

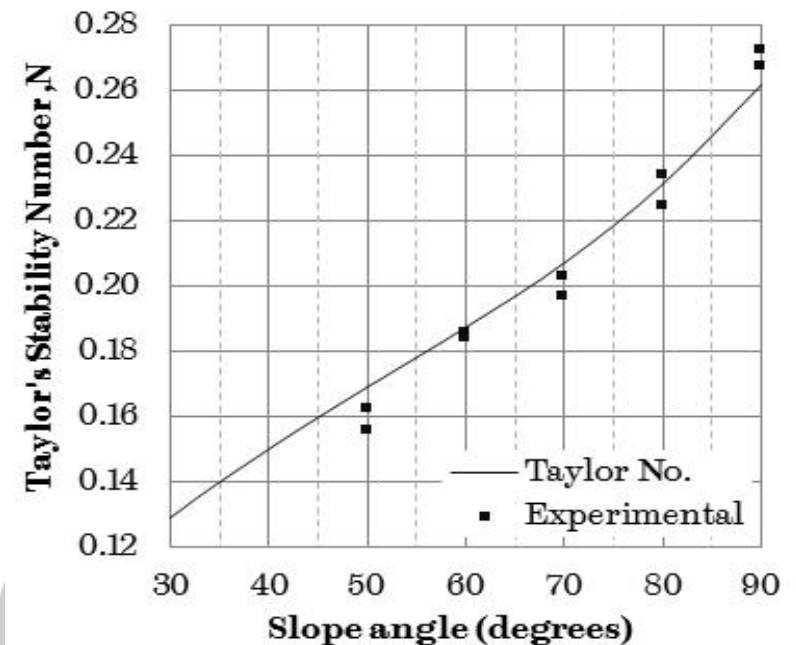
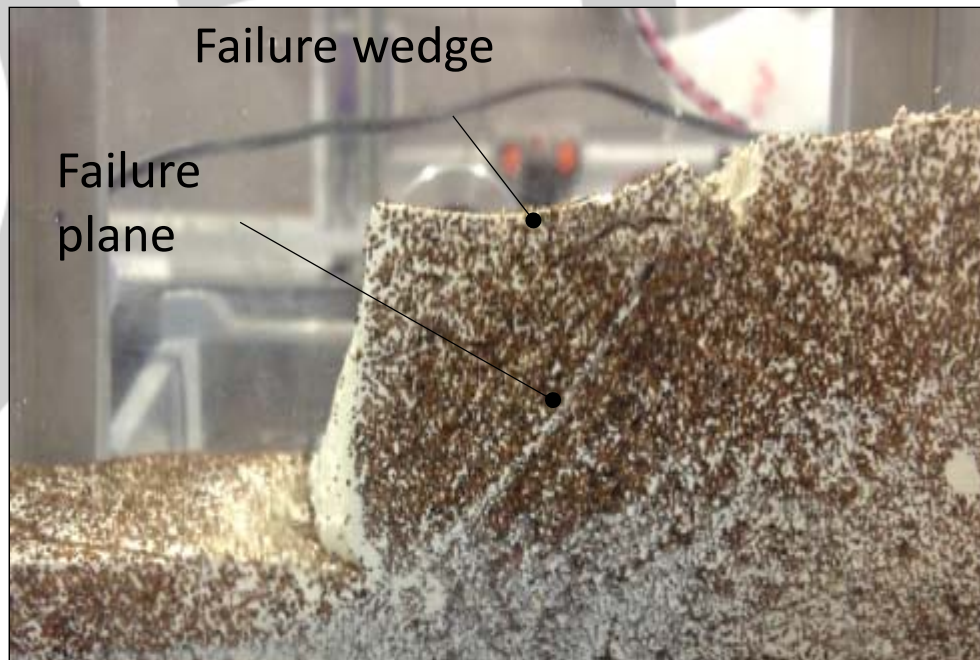
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Gravity switch on

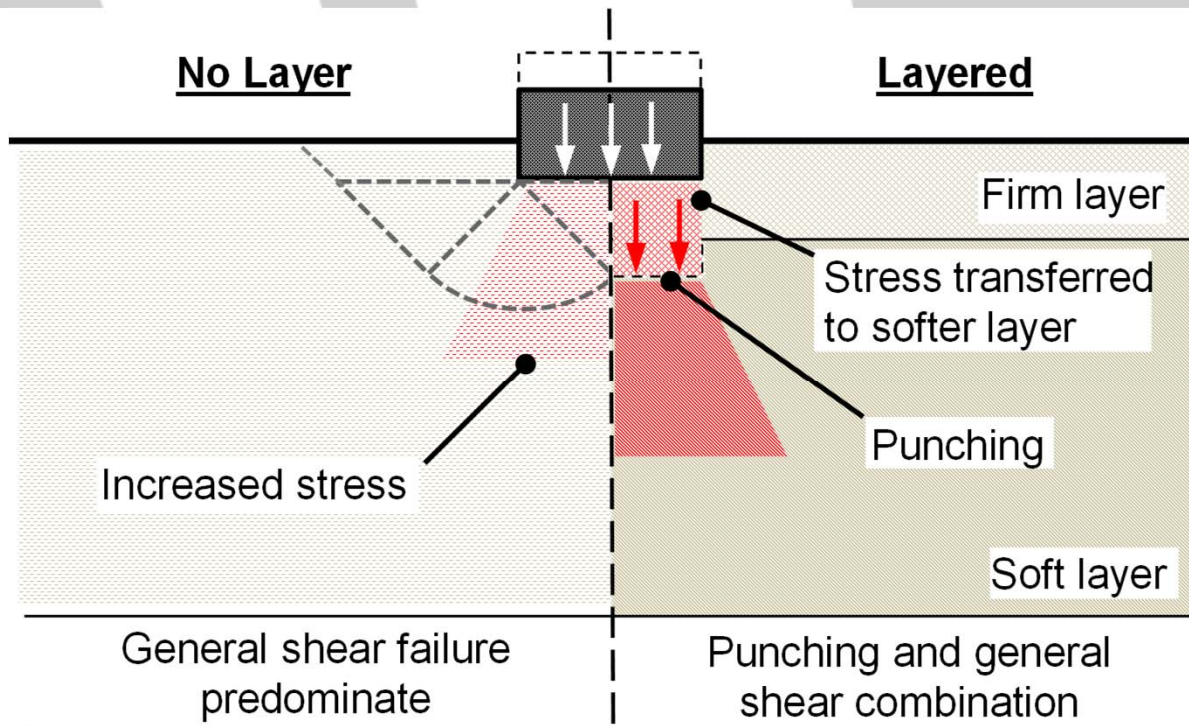
- Slope stability
- Comparison with limit equilibrium & empirical methods
- lope angle and soil strength considered





Shallow Foundation Project

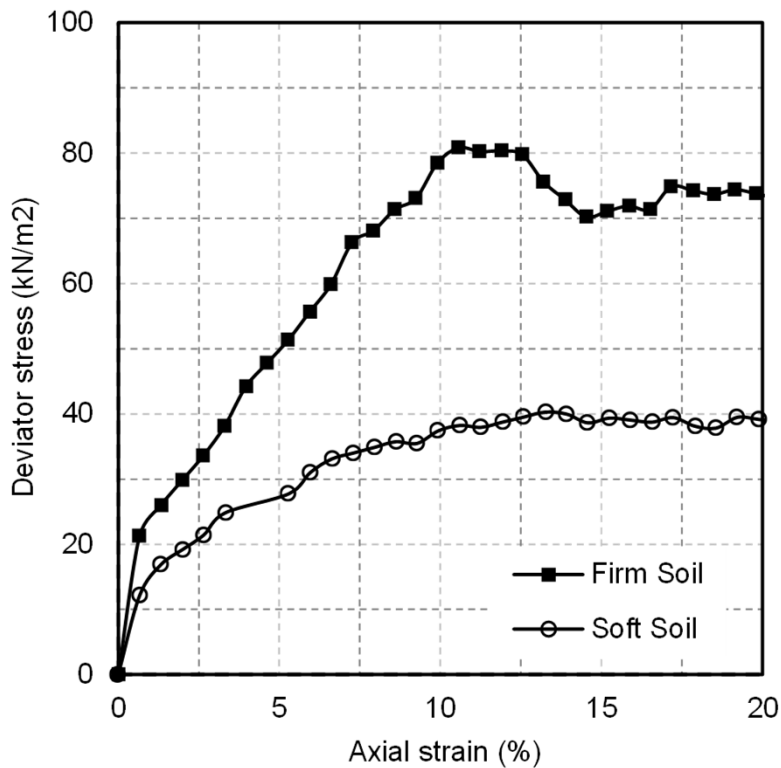
- Shallow bearing capacity of layered soils





Shallow Foundation Project

- Clay pre-consolidated at 400kN/m² & 200 kN/m²; $c_u = 40$ & 20 kN/m²



Test No.	Layer 1		Layer 2	
	H1 mm: [*m]	c_u1 kN/m ²	H2 mm: [*m]	c_u2 kN/m ²
1	80 [4.0]	40	0	N/A
2	40 [2.0]	40	40 [2.0]	20
3	20 [1.0]	40	60 [3.0]	20
4	15 [0.75]	40	65 [3.25]	20
5	10 [0.5]	40	70 [3.5]	20

Note: *prototype for acceleration N = 50g
Footing Width: B = 20mm [1.0]

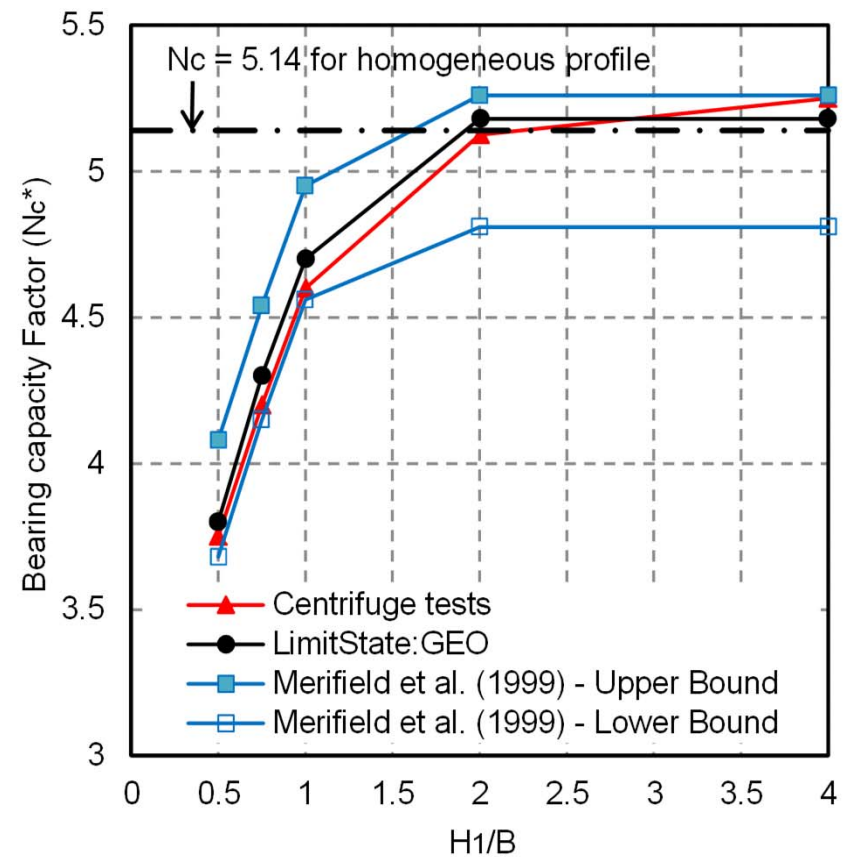
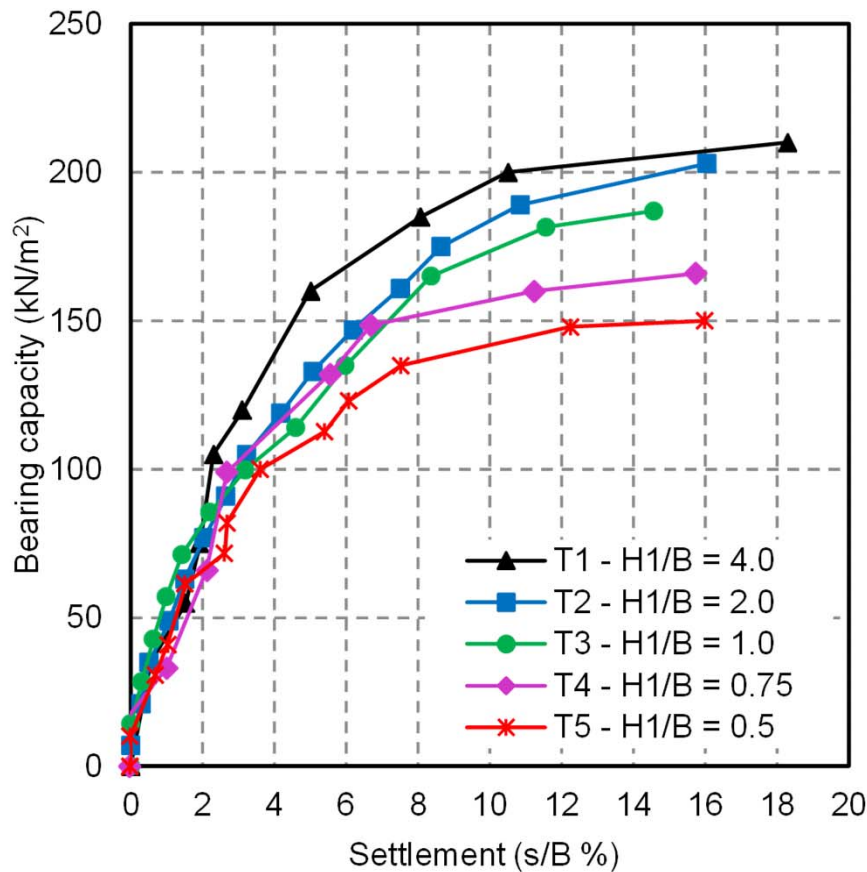




Bearing Capacity response:



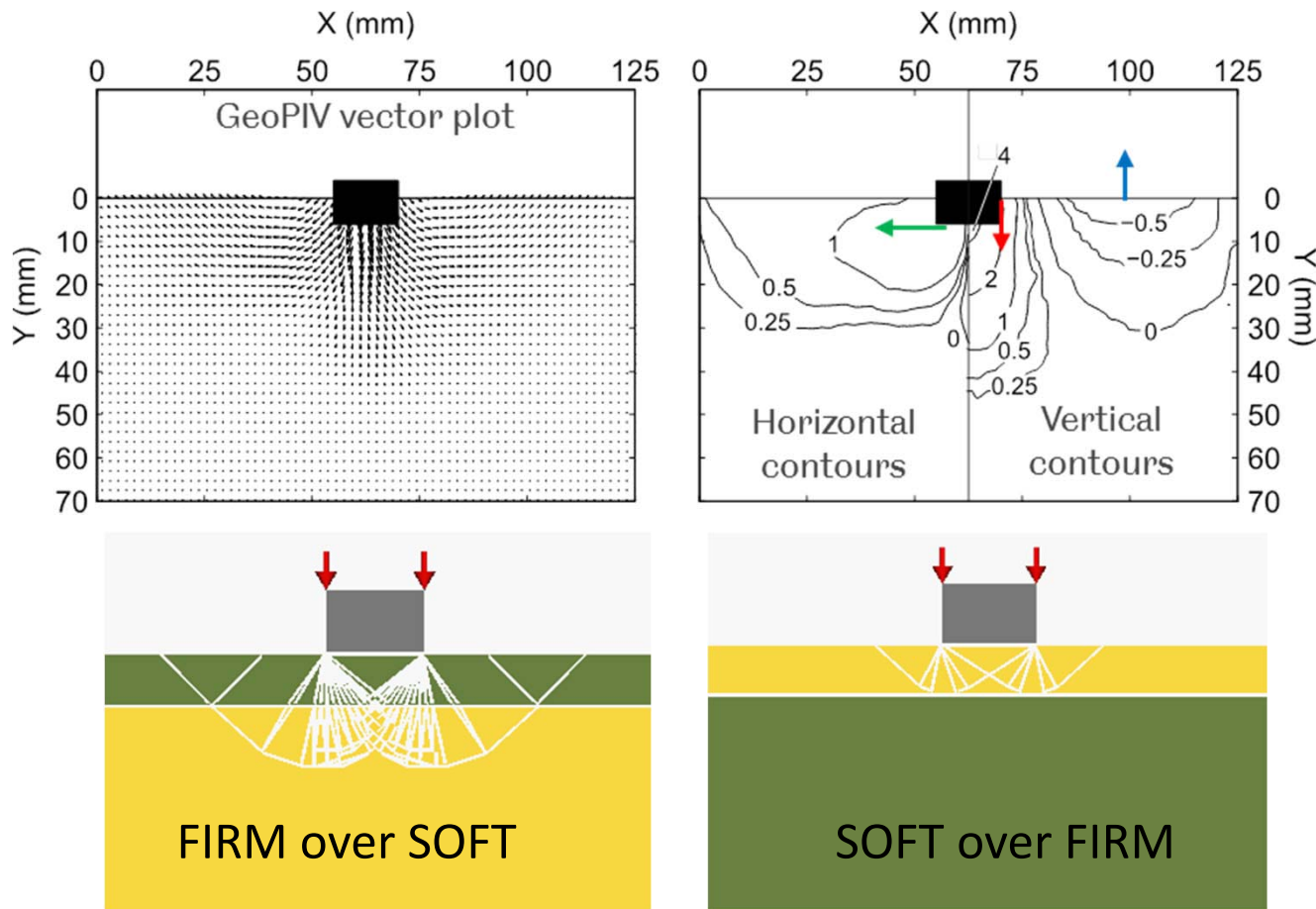
<http://www.limitstate.com/geo>





Performance compared to:

- GeoPIV soil velocity and displacement mechanics





Effects of void formation on bending response of buried flexible utility pipes.

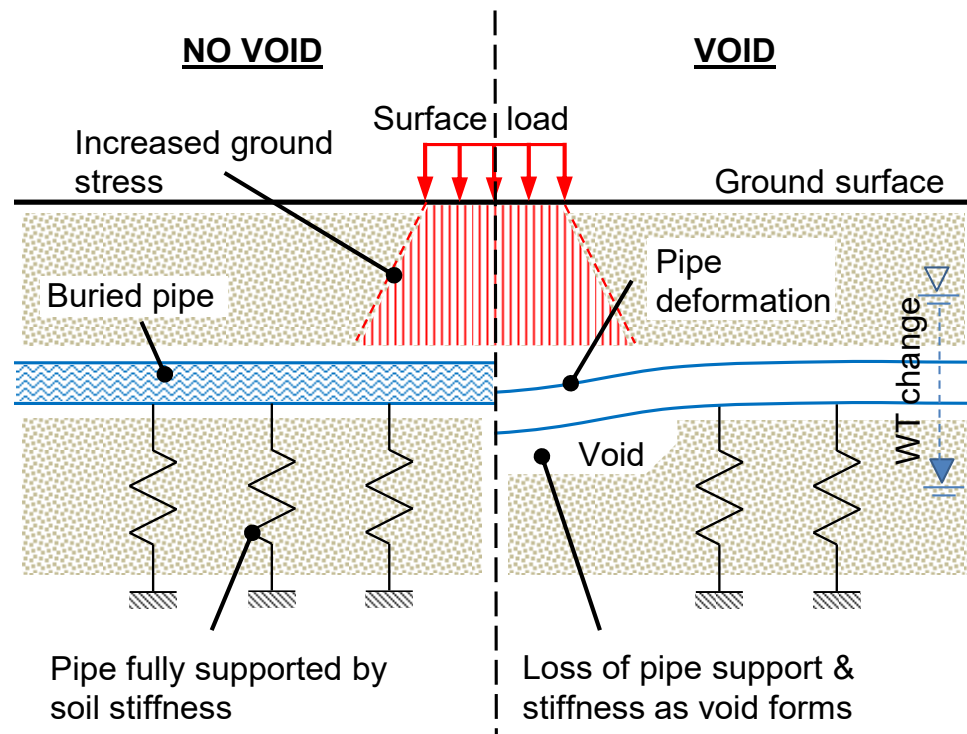
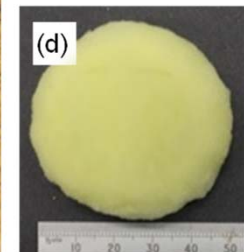
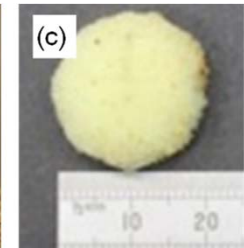
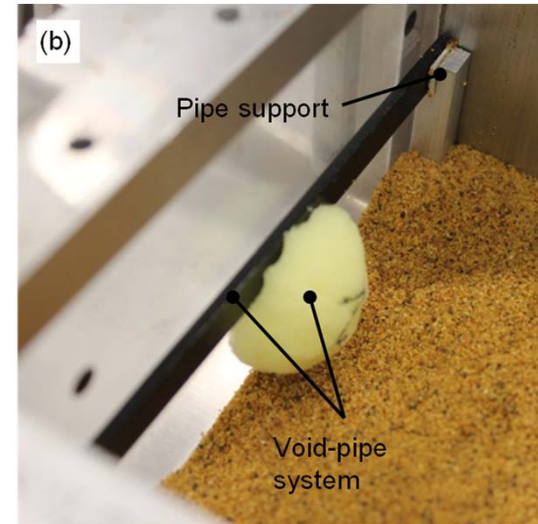
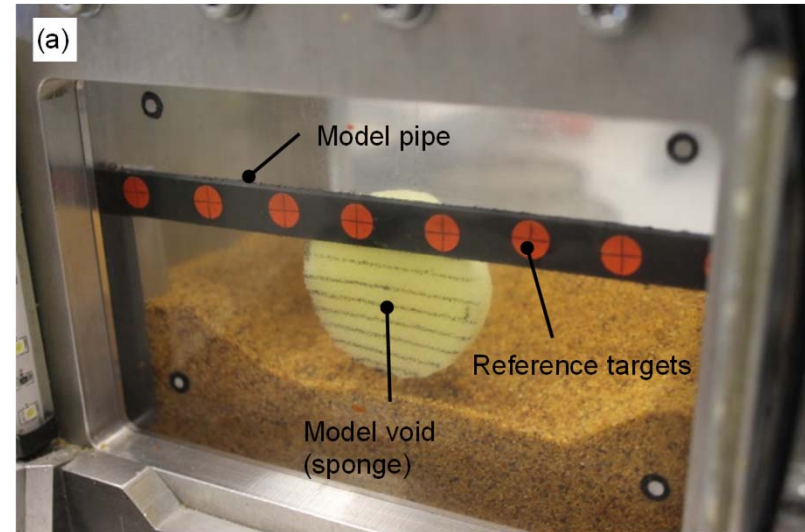
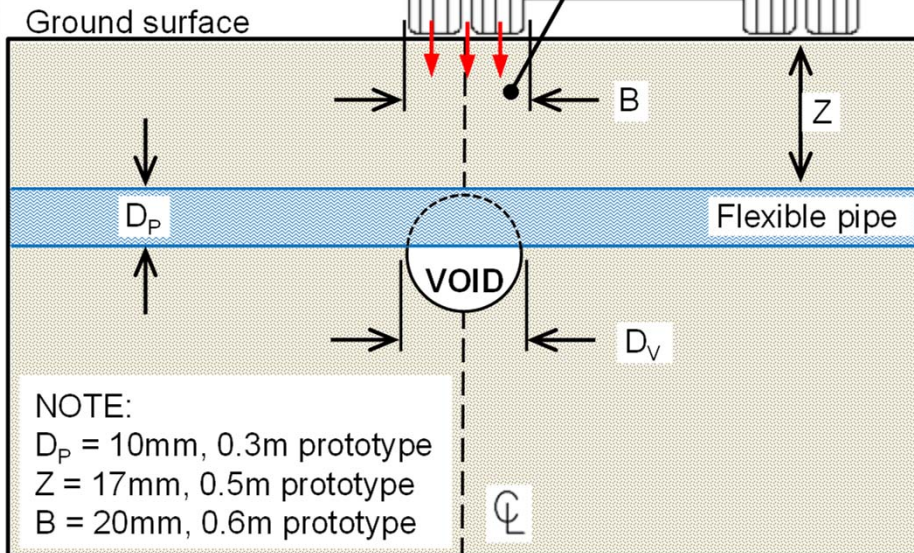
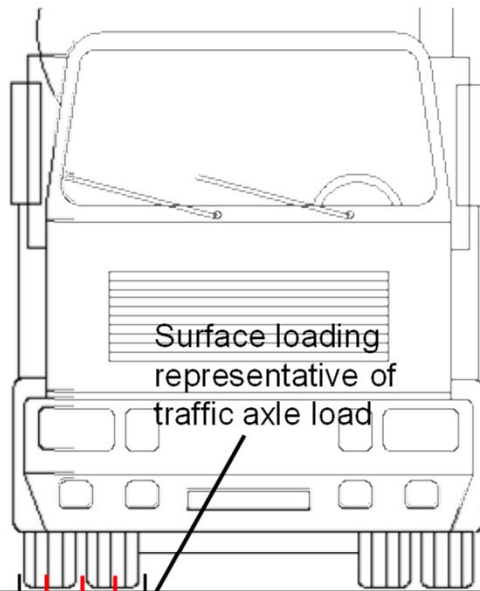




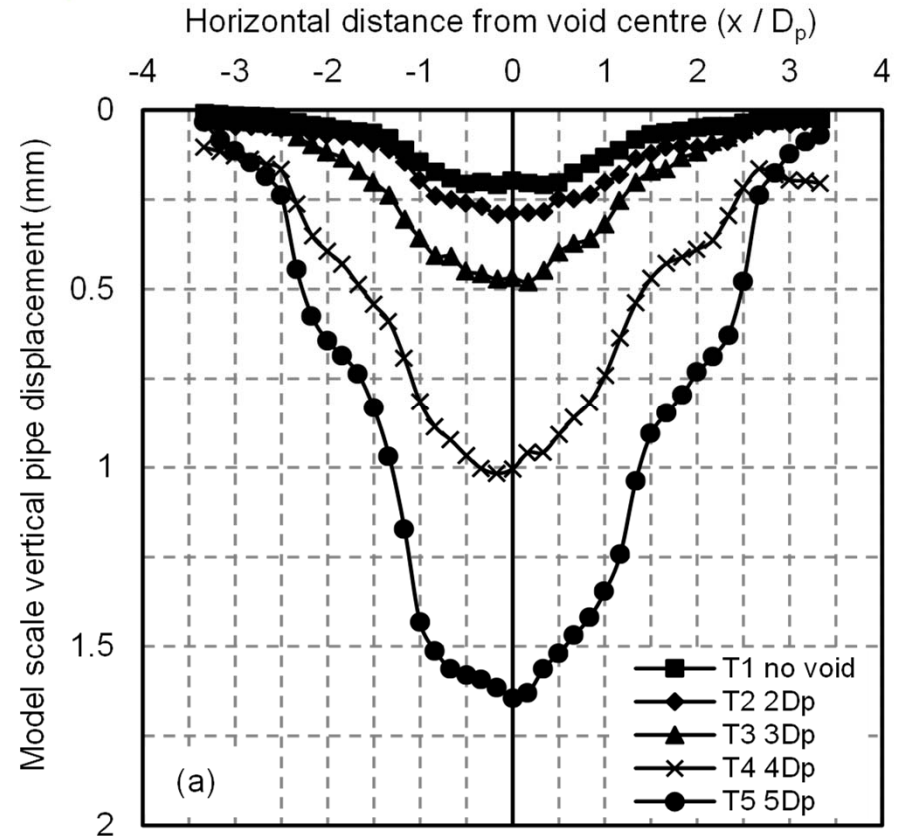
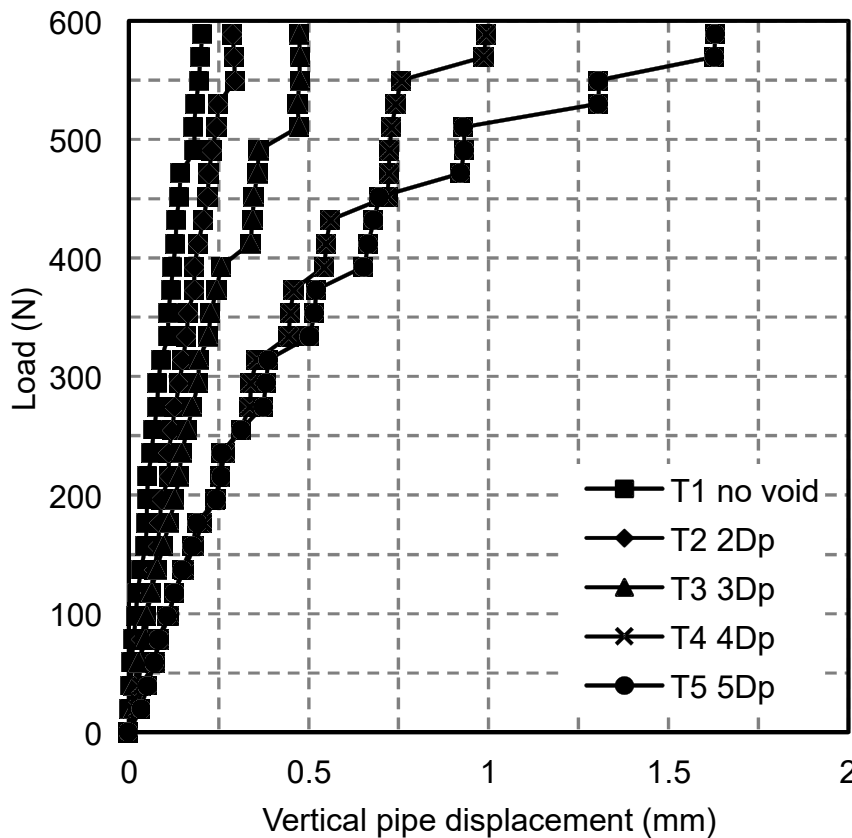
Table 3. Test Info.

	Pipe	$D_v (\emptyset)$
1	Yes	None
2	Yes	$2 D_p$
3	Yes	$3 D_p$
4	Yes	$4 D_p$
5	Yes	$5 D_p$





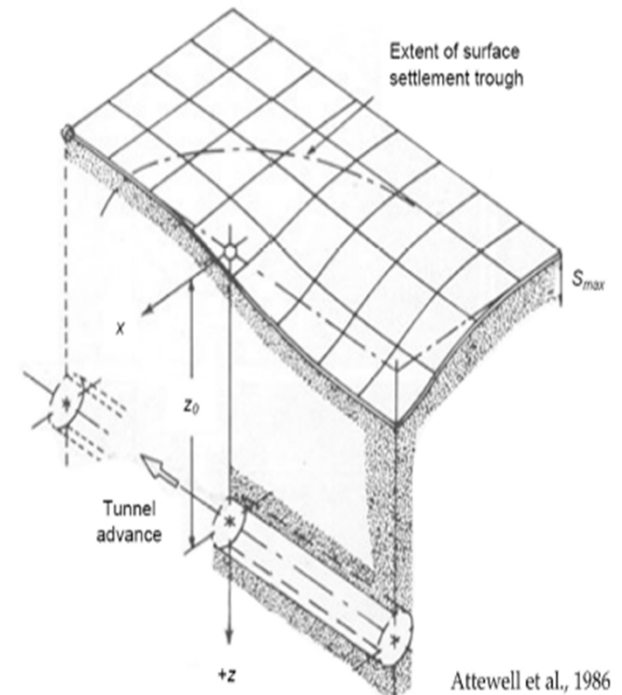
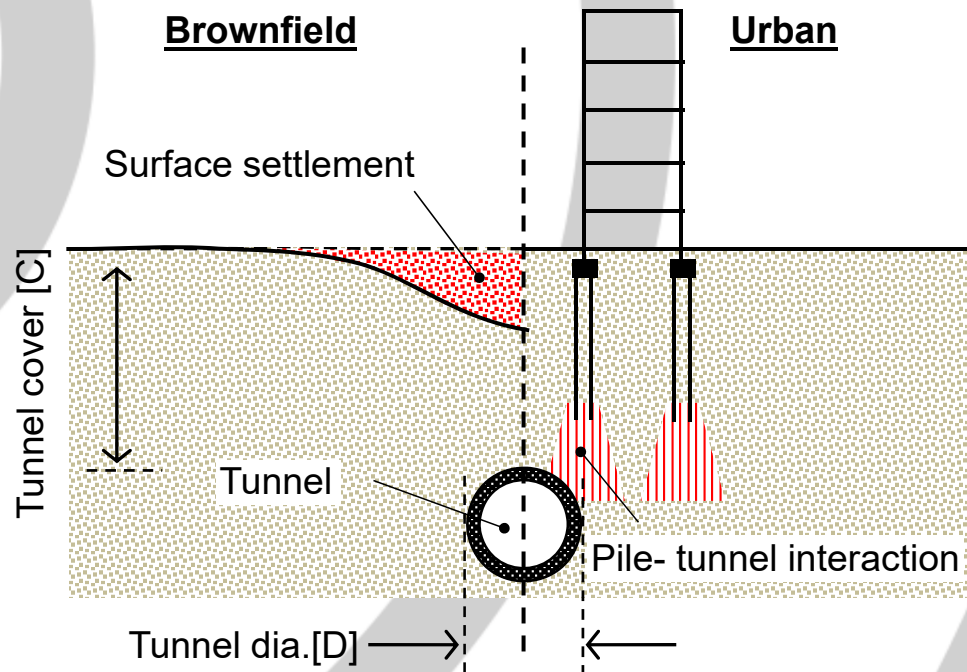
Pipe displacement & centre of void position





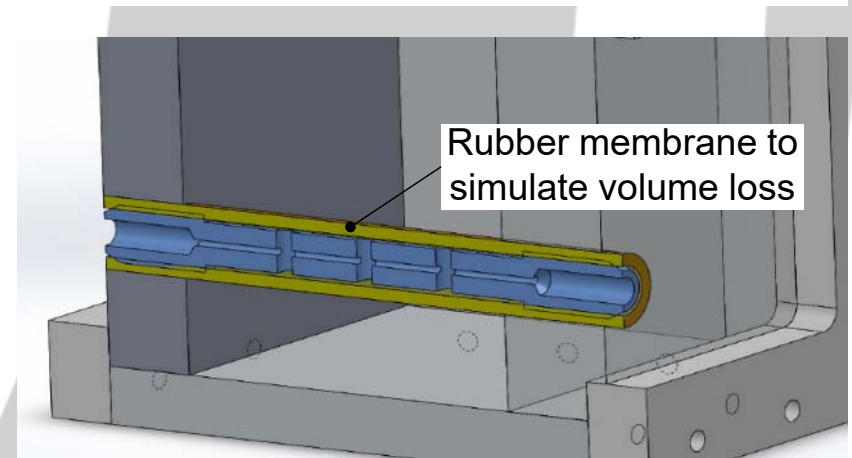
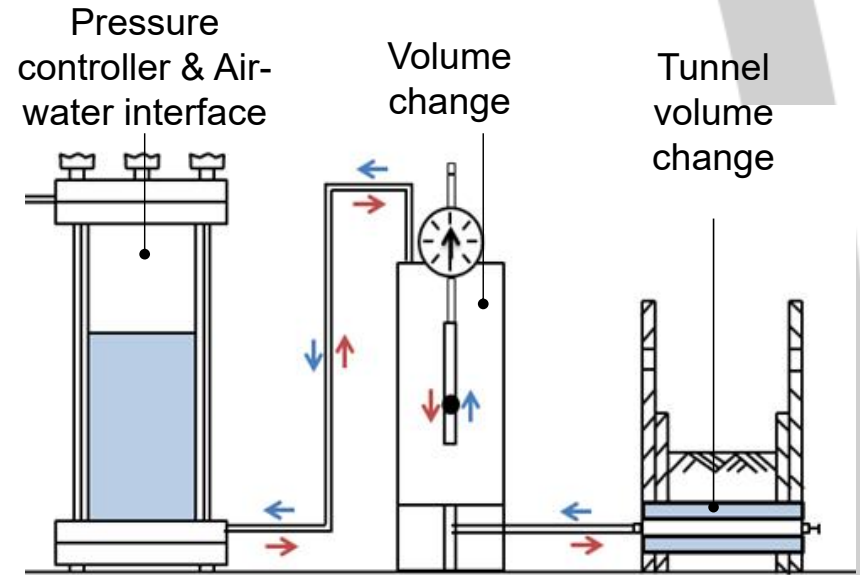
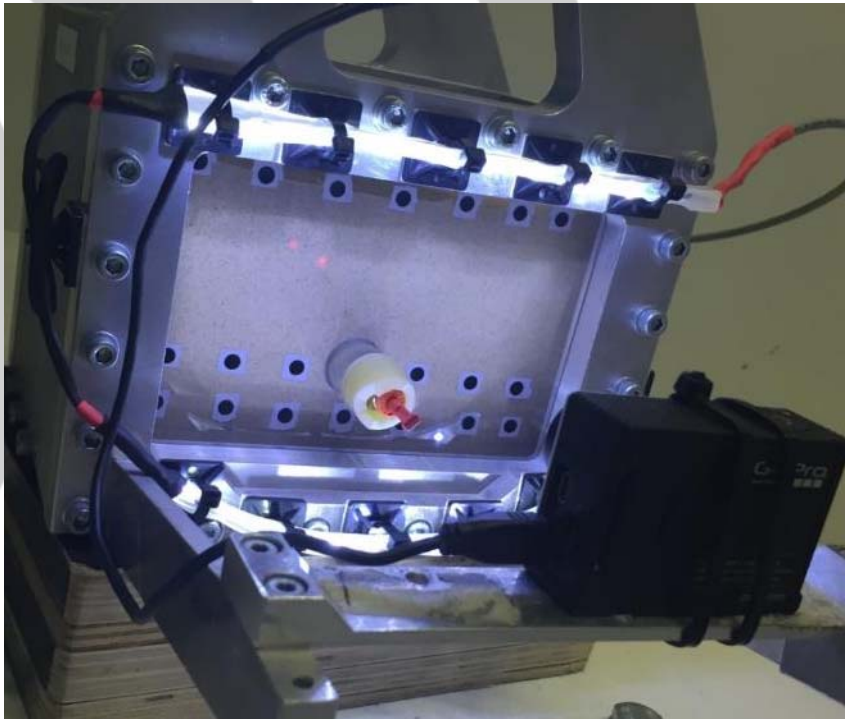
Impact of tunnelling on buried infrastructure

- Verification of surface settlement
- Interaction with existing infrastructure





Tunnel Vol. loss simulation

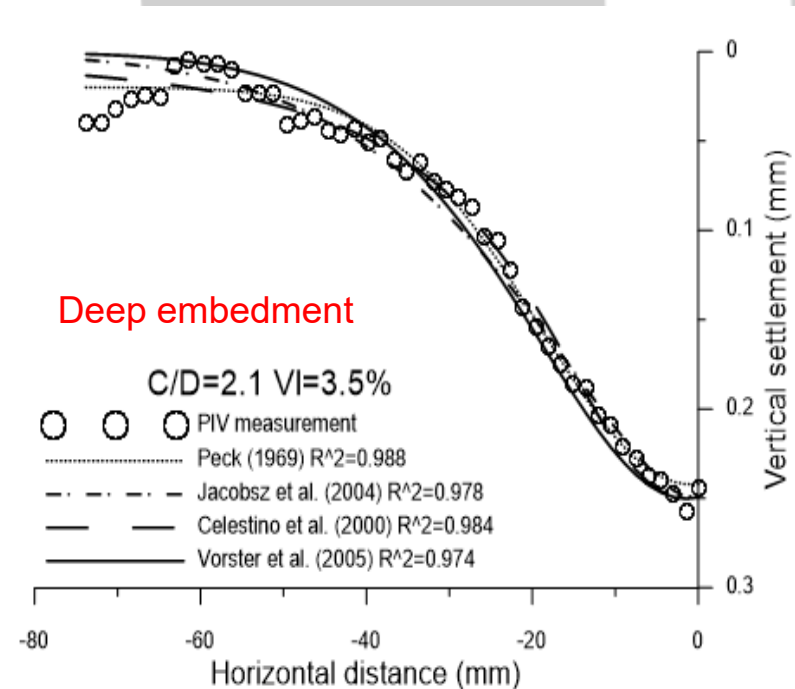
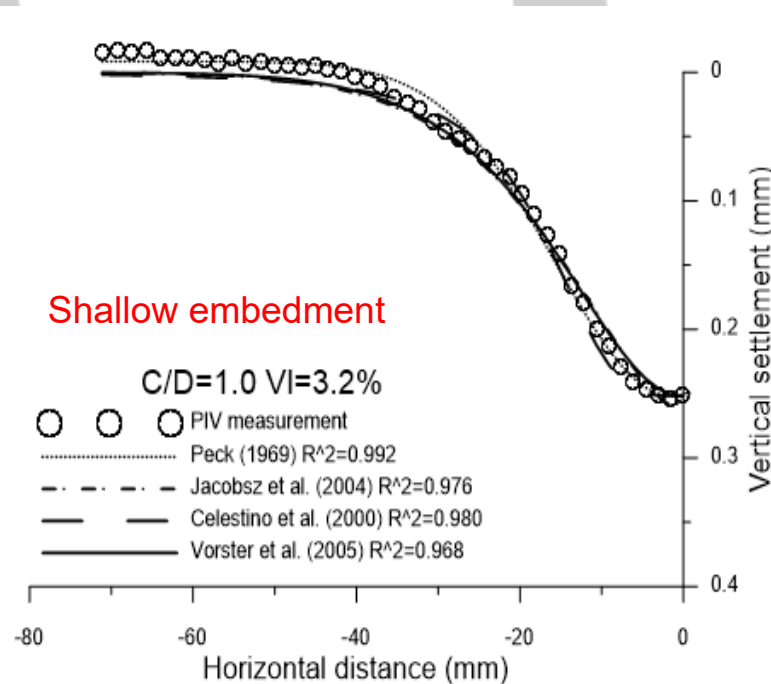




Impact of tunnelling on buried infrastructure

- Tunnel diameter to cover ratio C/D = 1.0; 1.6 and 2.1
- Comparison to classic theory [Peck 1969] & recent works

$$S_v(x) = S_{max} \exp\left(\frac{x^2}{2i^3}\right)$$





Lessons to remember

1. **Keep it simple**
2. **Keep it simple!**
3. Allow students to make mistakes & learn from failures
4. Integration of preparation systems vital for throughput
5. Good things do come in small packages
6. **Remember to keep it simple !**





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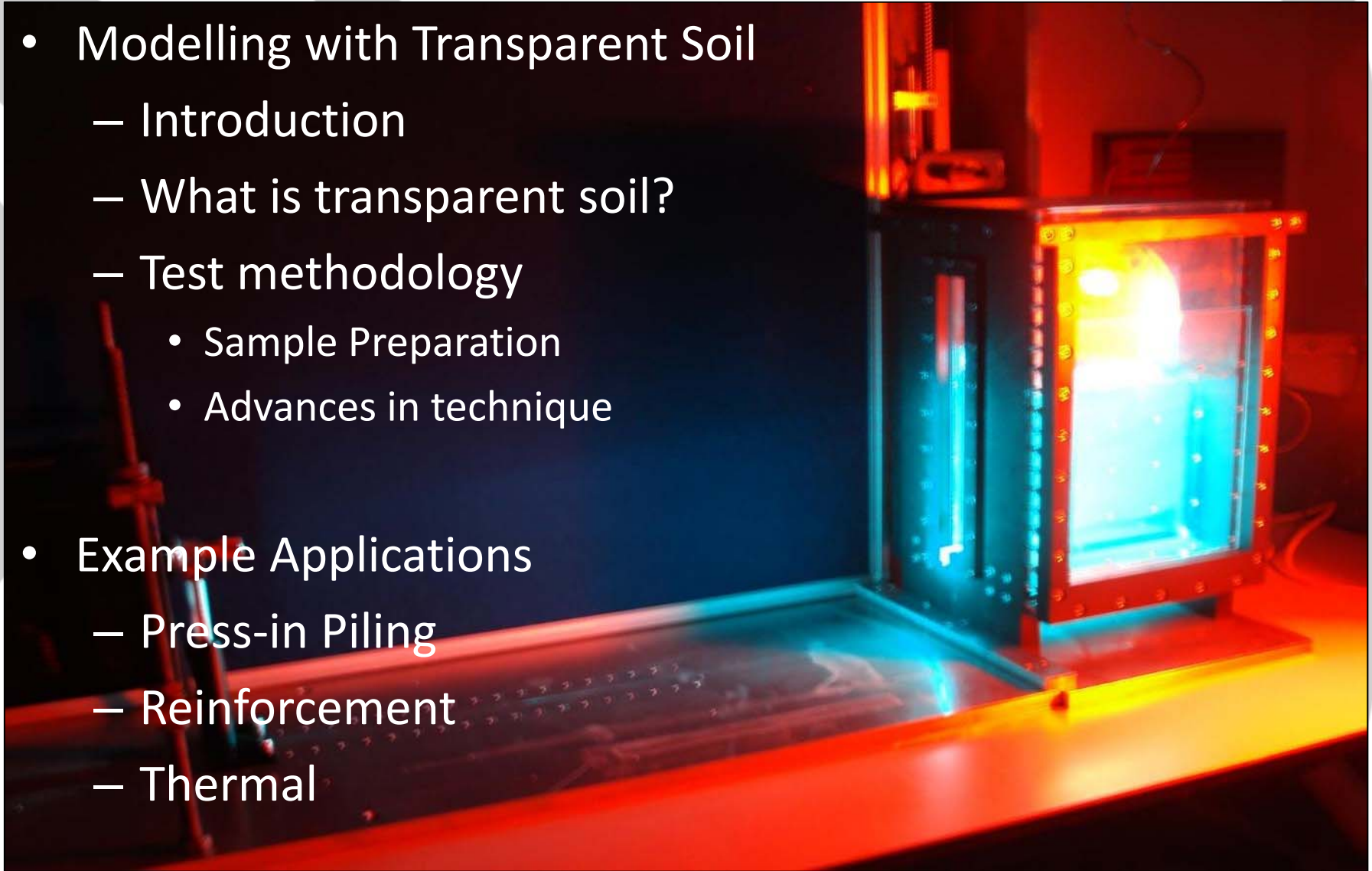
Impact to student learning

- Observation & validation of theory
- Connecting analytical, numerical, empirical methods
- Developing experimental research skills
- Promoting geotechnical research
- Test facility for scoping out larger studies





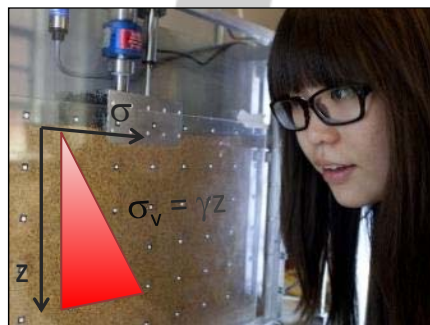
- Modelling with Transparent Soil
 - Introduction
 - What is transparent soil?
 - Test methodology
 - Sample Preparation
 - Advances in technique
- Example Applications
 - Press-in Piling
 - Reinforcement
 - Thermal



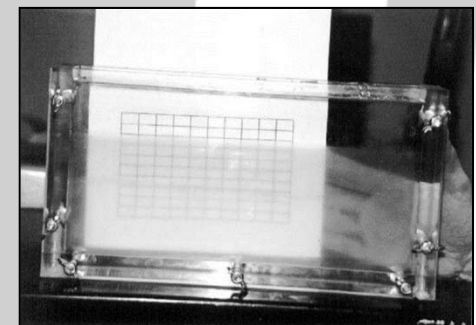


- Visualisation used extensively to gain greater insight
 - Medical, engineering, environmental
- Desire to visualize soil deformation to enhance understanding - interaction, failure mode, design codes
 - X-ray: discrete lead shot, low resolution, compliance errors
 - Photogrammetry: *Plane Strain* modelling – Stereo, PIV ‘GeoPIV’
 - Limitations: Boundary effects, geometry & installation simplification

Plane Strain

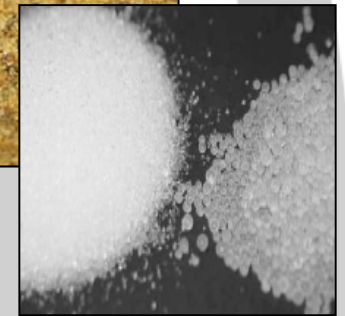
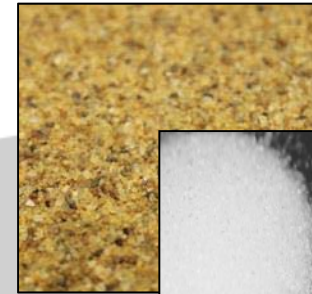


Non-intrusive





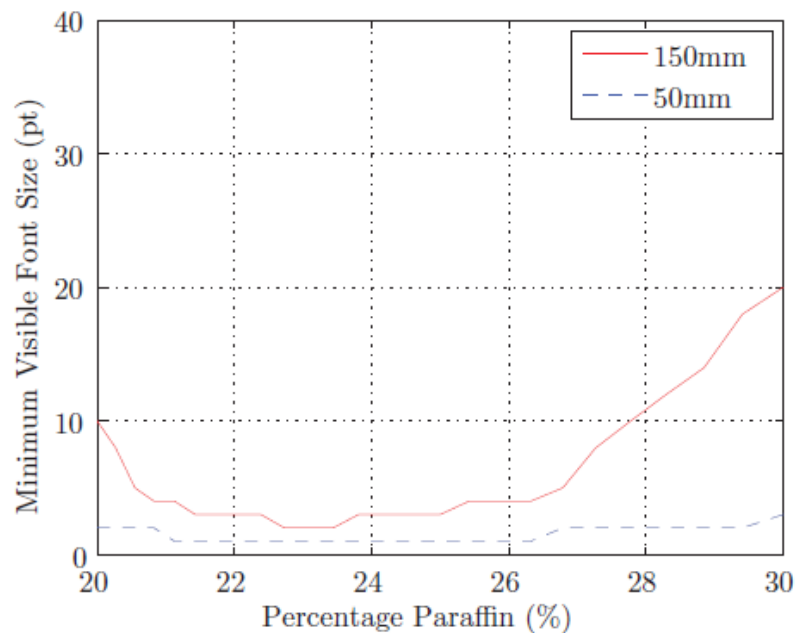
- Transparent 'soil' development
 - Range of analogue soil materials adopted
 - Coarse: glass, silica gels, fused quartz
 - Fine: Precipitated & Fumed amorphous silica
 - Aggregate & matched refractive index pore fluid
 - Blended mineral oils
 - Two phase (remove air)
 - Optical quality
 - Aggregate dependant
 - Oil quality & calibration
 - Temperature dependent
- Impact on sample size possible





- Optical quality assessment
 - Eye test chart: ‘Snellen chart’ at opticians
 - Located behind specimen, ‘say what you see’
 - Calibration of optimum material blend
 - Subjective & operator dependent
 - Test chamber geometry

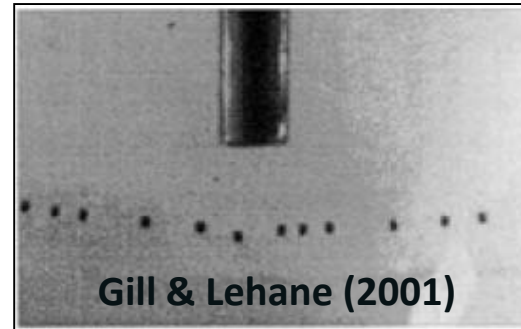
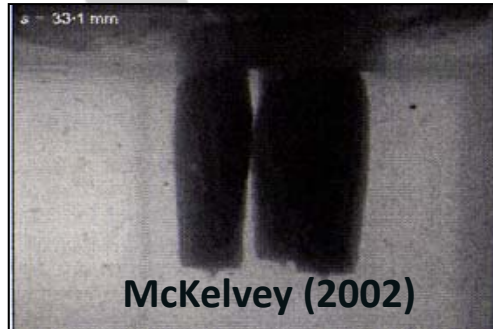
Test Card Example	Point Size
54521	72
21036	48
27147	36
78828	28
61638	26





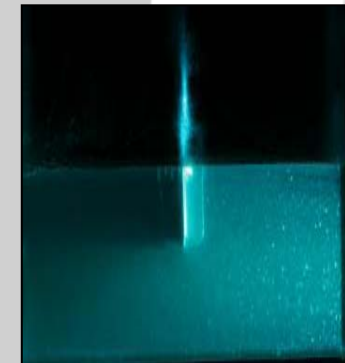
- Methods of observation in transparent soil

Silhouette tracking



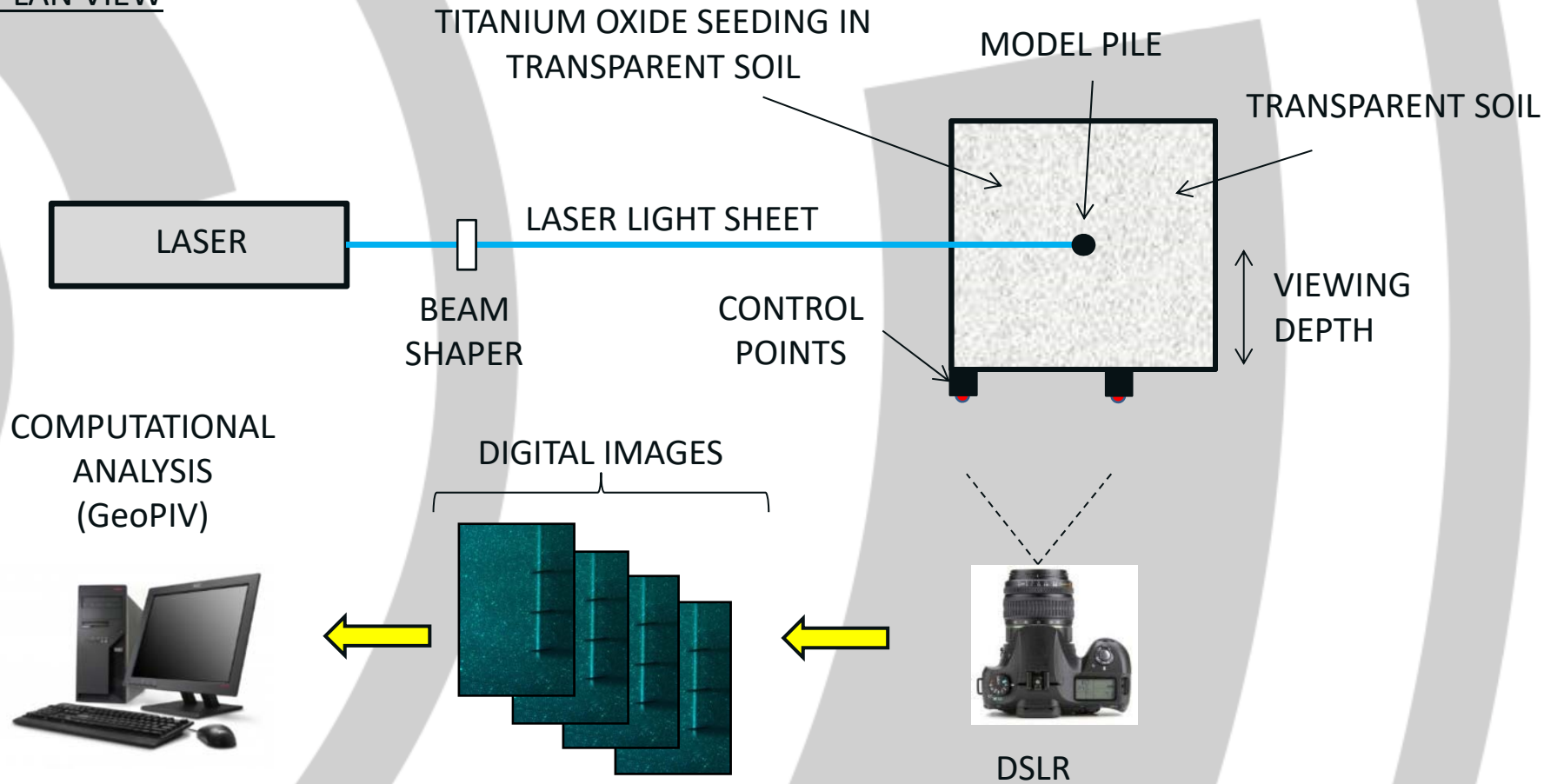
Target beads

- Material quality not as critical in all cases
 - Low resolution, compliance errors from inclusions
- How to measure/record internal high-resolution displacements..?
- PIV similar to fluid mechanics
 - Tracking of seeding particles
 - Laser aided imaging
 - Image correlation



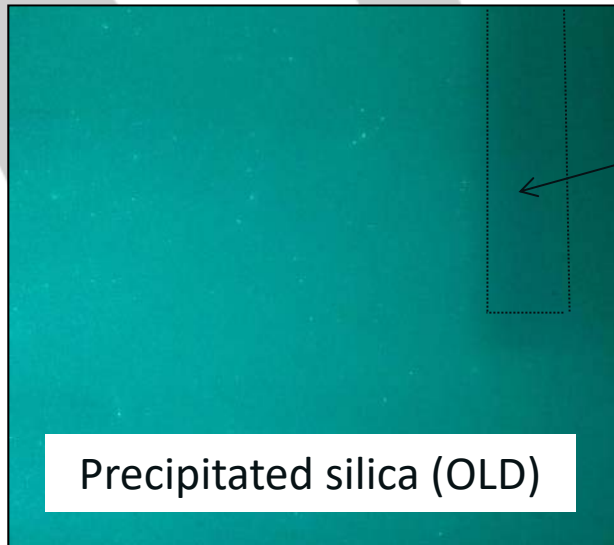


PLAN VIEW



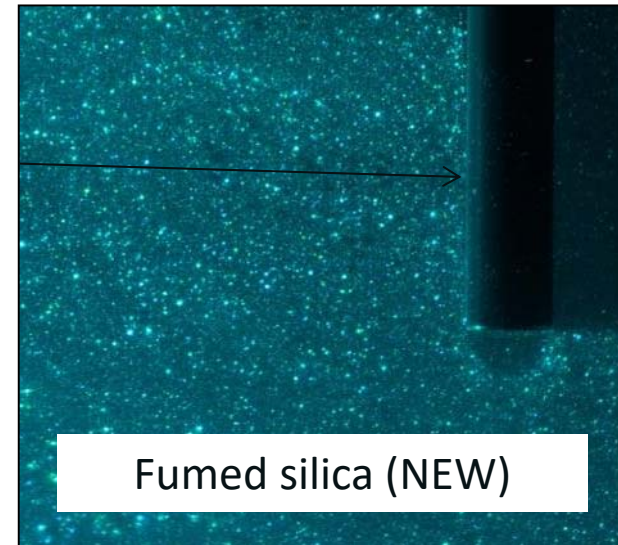


- Laser aided imaging illuminates 'soil' texture



Precipitated silica (OLD)

12 mm
diameter
model pile



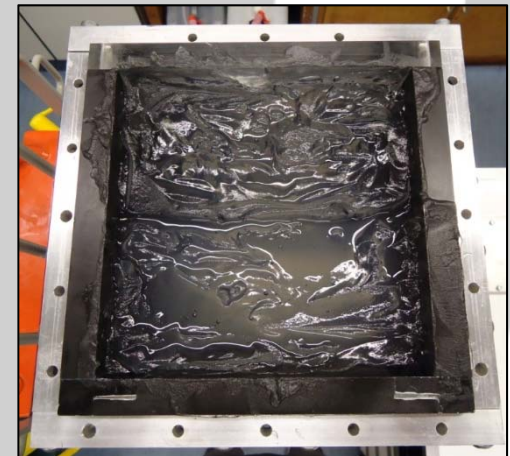
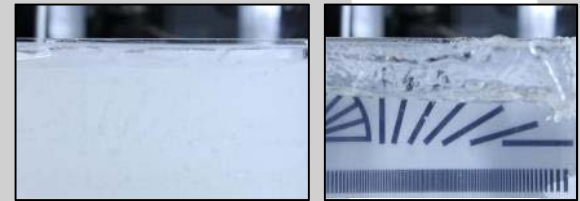
Fumed silica (NEW)

- Impact of soil type & oil quality on optical clarity
 - Comparison above @ 100mm viewing depth
 - Precipitated: Iskander et al. 1994, Hird et al. 2008, Ni et al. 2010
 - Fumed: New UoS formulation of aggregate and oil



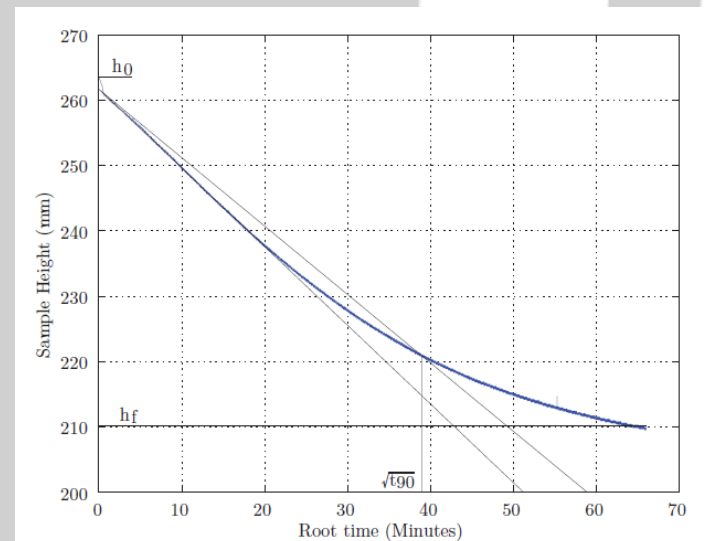
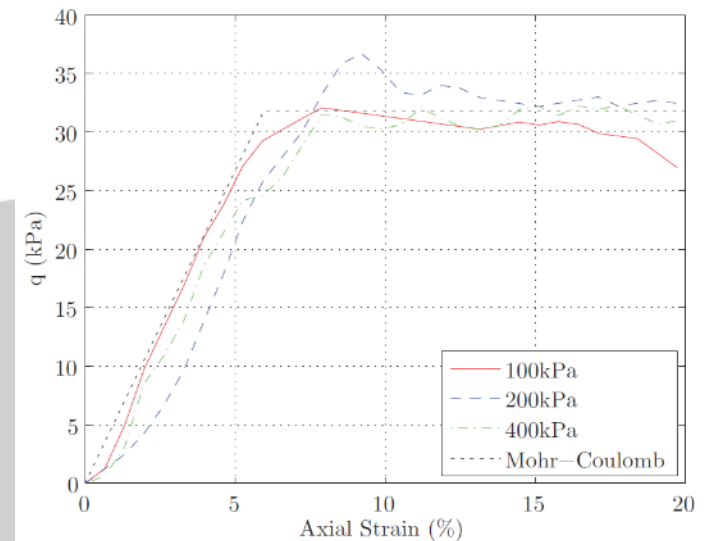


- Transparent Soil: UoS formulation
 - Aggregate – fumed silica ($D_{50}=14\mu\text{m}$)
 - Pore fluid – matched Refractive Index
 - White oil (77%), Paraffin (23%)
 - Seeding texture for PIV analysis
 - Titanium Oxide particles ($D_{50}=20\mu\text{m}$)
 - In-lieu of discrete targets i.e. beads
- Sample preparation:
 - Mixed in slurry form
 - De-aired under vacuum
 - Consolidated to desired stress



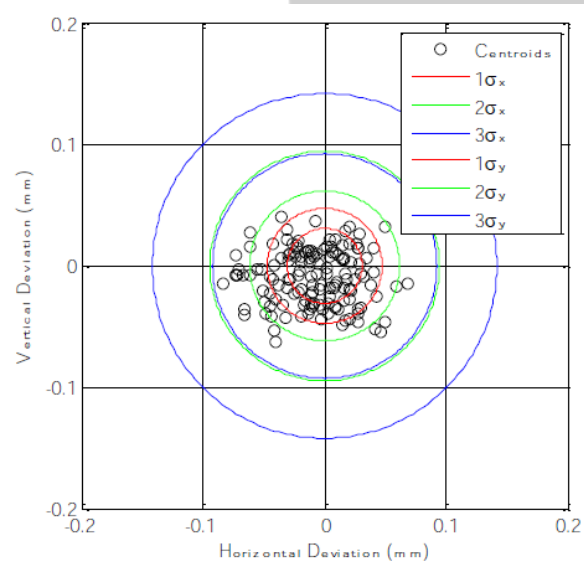
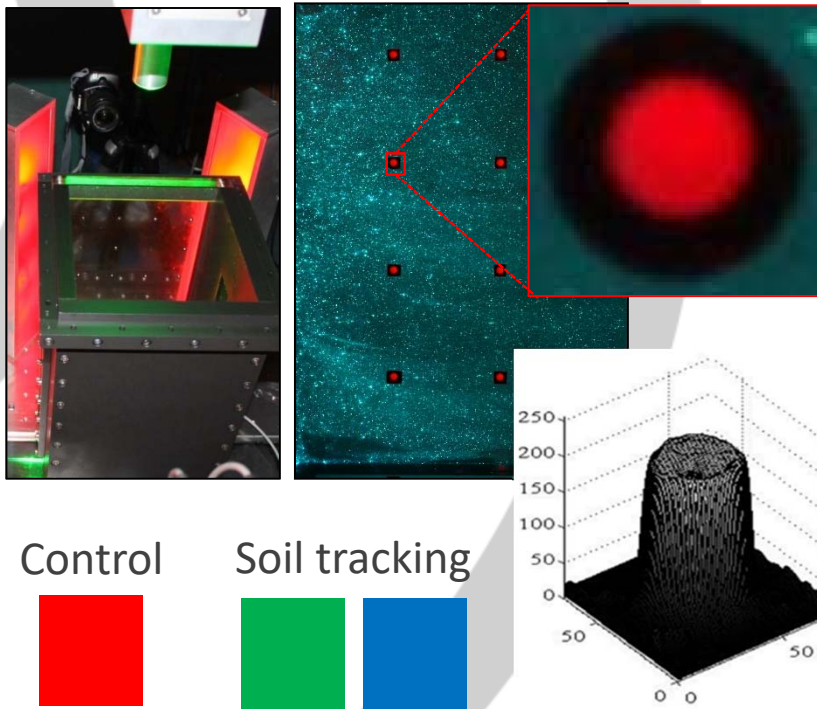


- Stress – strain response
 - Consolidated to 100kPa
 - Undrained shear strength $\sim 15\text{-}20\text{kPa}$
 - Coeff. of consolidation = $3\text{-}4 \text{ m}^2/\text{year}$
- Problems that can arise
 - Air entering the model
 - Cavitation during unloading
 - Positive pour fluid head maintained

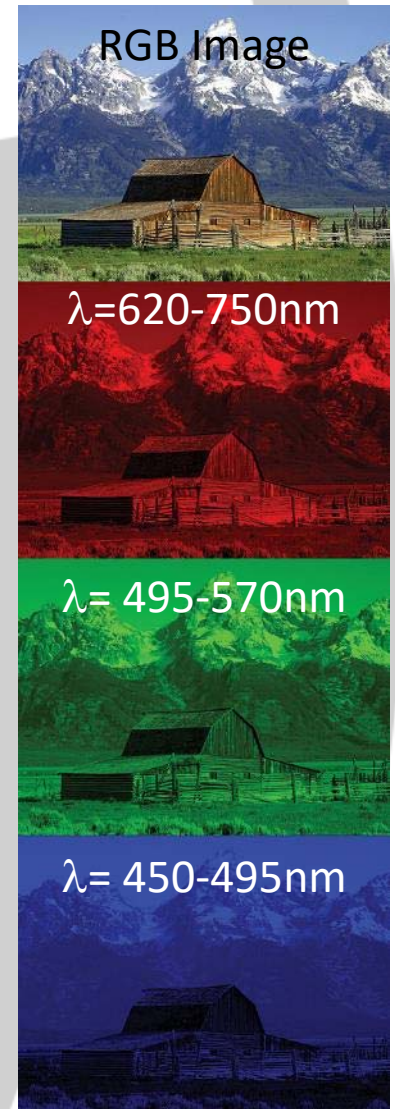




- New control target markers
 - Image contains Red: Green: Blue colour channels
 - Laser wavelength of 532nm (GREEN)
 - Red light channel used for the control markers
 - Soil displacement conducted on Blue & Green

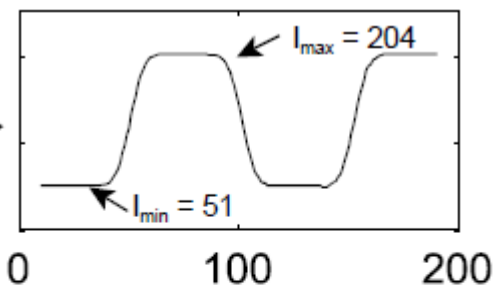
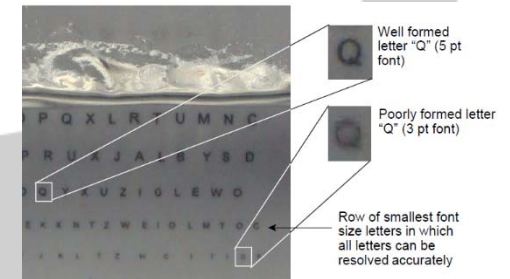


Factor of 3 reduction in control marker error





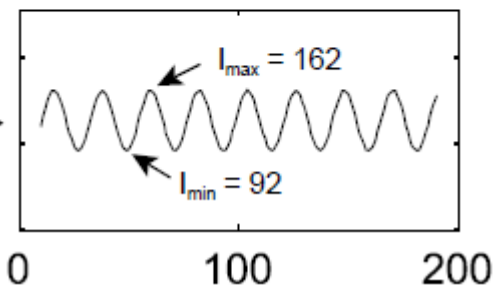
- New framework for assessing transparency
 - Eye chart not sufficiently robust
- Modulation Transfer Function (MTF)
 - Standardised method of lens assessment
 - *'measures how faithfully the lens reproduces (or transfers) detail from the object to the image produced by the lens'*



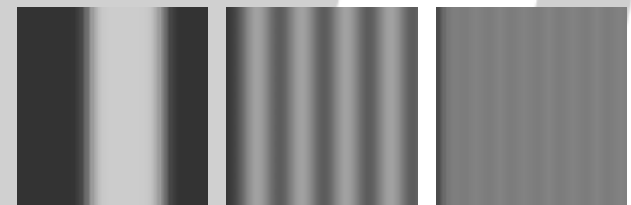
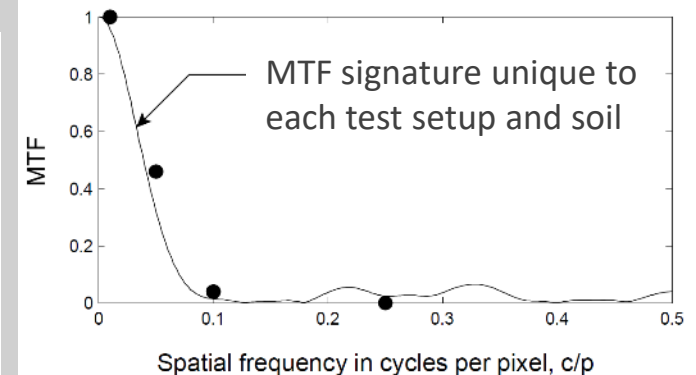
Modulation of actual image

$$\frac{(204-51)/(204+51)}{(204-51)/(204+51)} = 1$$

Modulation of ideal image



$$\frac{(162-92)/(162+92)}{(204-51)/(204+51)} = 0.46$$





- Tubular or square shaft
- Single or regularly spaced helical plates
 - Typically 100-400mm diameter
- Minimal ground disturbance
- Recoverable/reusable
- Typical applications:
 - Marine anchors
 - Road and rail signage
 - Oil and gas pipelines
 - Temporary structures

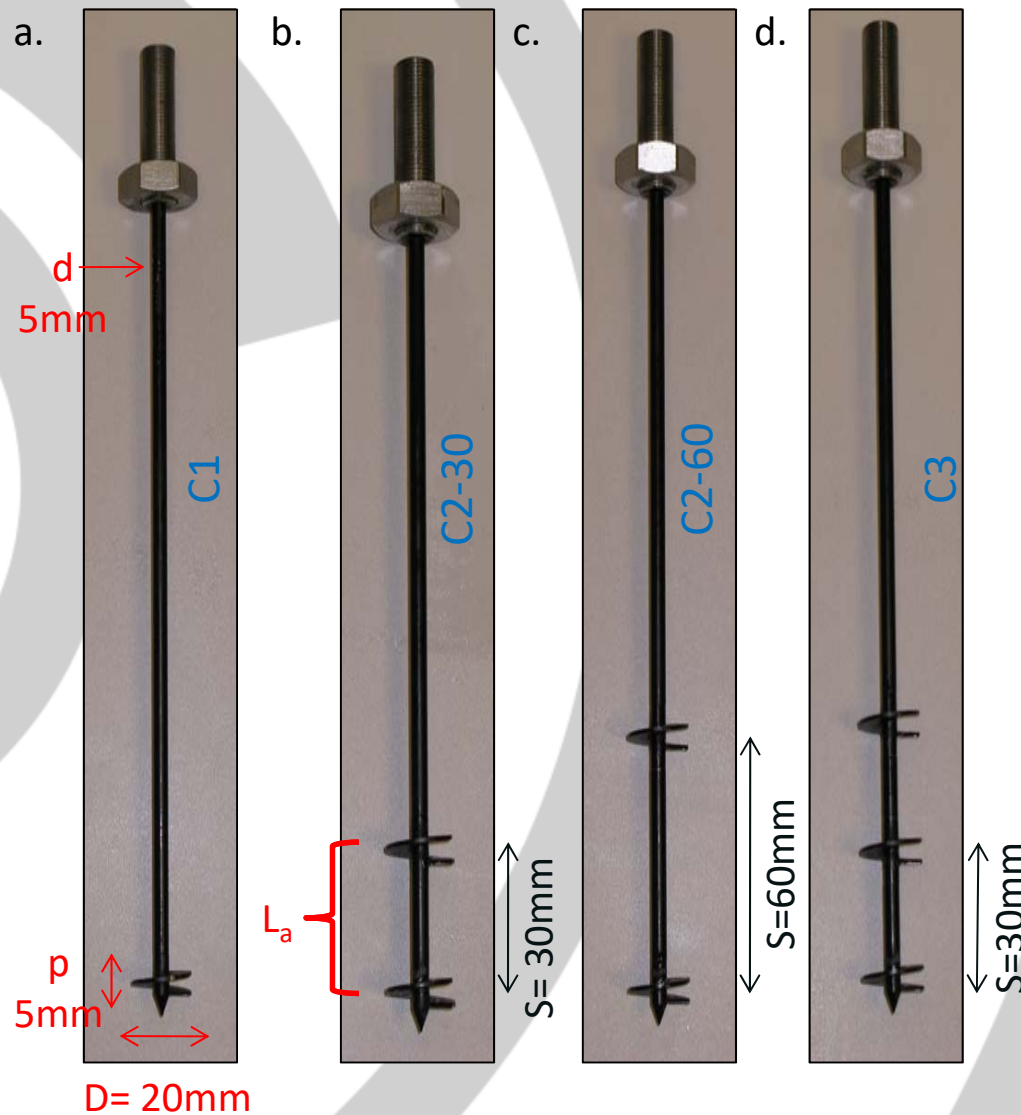
Failure mechanics not definitively known





Modelling with Transparent Soil

Helical screw piles: model piles



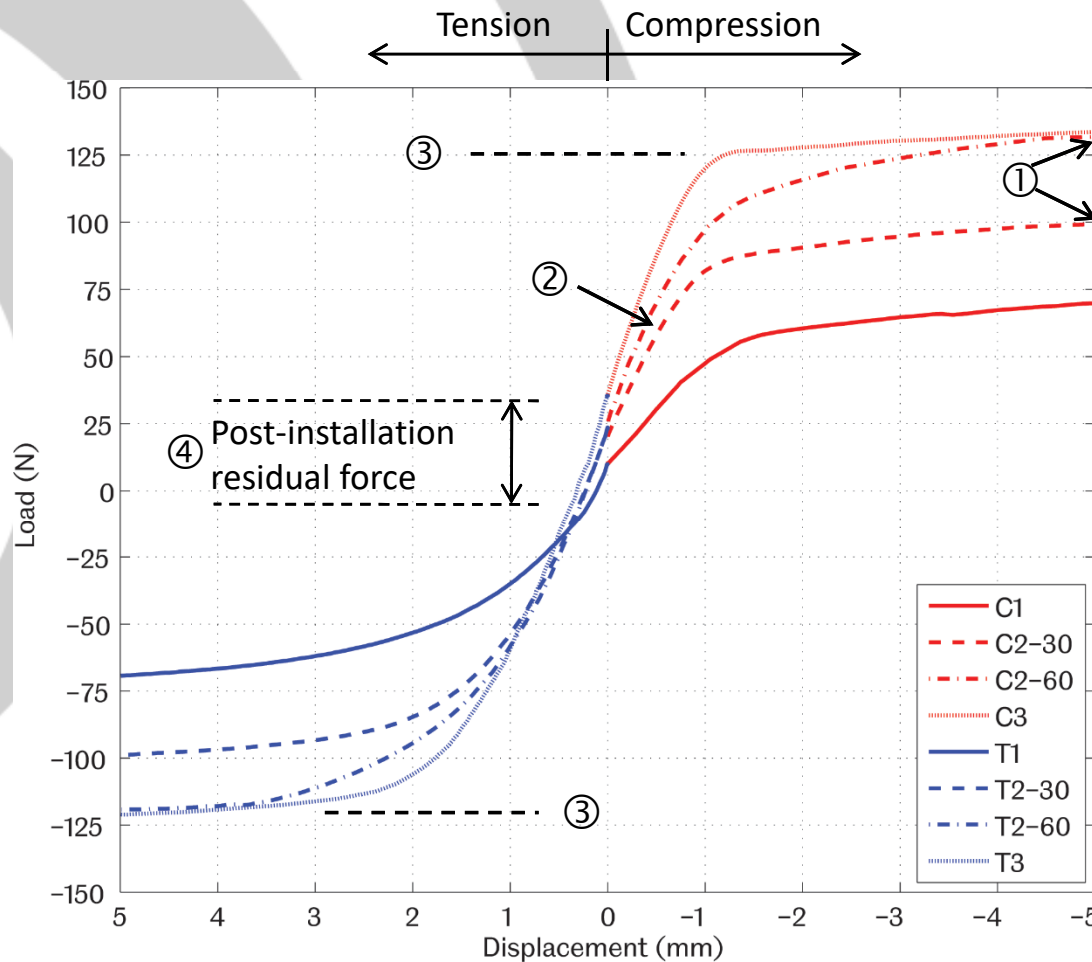
- Machined aluminum piles (1:10)
- Plate diameter (D) = 20mm
- Flight pitch (p) = 5mm
- Shaft diameter (d) = 5mm
- Configurations (spacing – s):
 - a. Single helix pile
 - b. Double helix pile: $s = 30$ mm
 - c. Double helix pile: $s = 60$ mm
 - d. Triple helix pile: $s = 30$ mm
- Active Length (L_a) between flights
- Screwed installation
- 0.2 mm/s loading (undrained rate)
- Deep embedment - $H > 4D$
- 24 hour setup period allowed after installation prior to loading





Modelling with Transparent Soil

Helical screw piles: load-deflection performance



1. Ultimate capacity governed by L_a

- C2-30 & C2-60
- C2-60 & C3; $L_a = 60\text{mm}$

2. Development of capacity governed by No. of plates & s/D

- $C3 > C2-60 > C1$
- Increases with No. of plates

3. Compressive and tensile capacity very similar

4. Post-installation residual force proportional to model pile volume

- i.e. number of helical plates

What are the failure mechanisms?

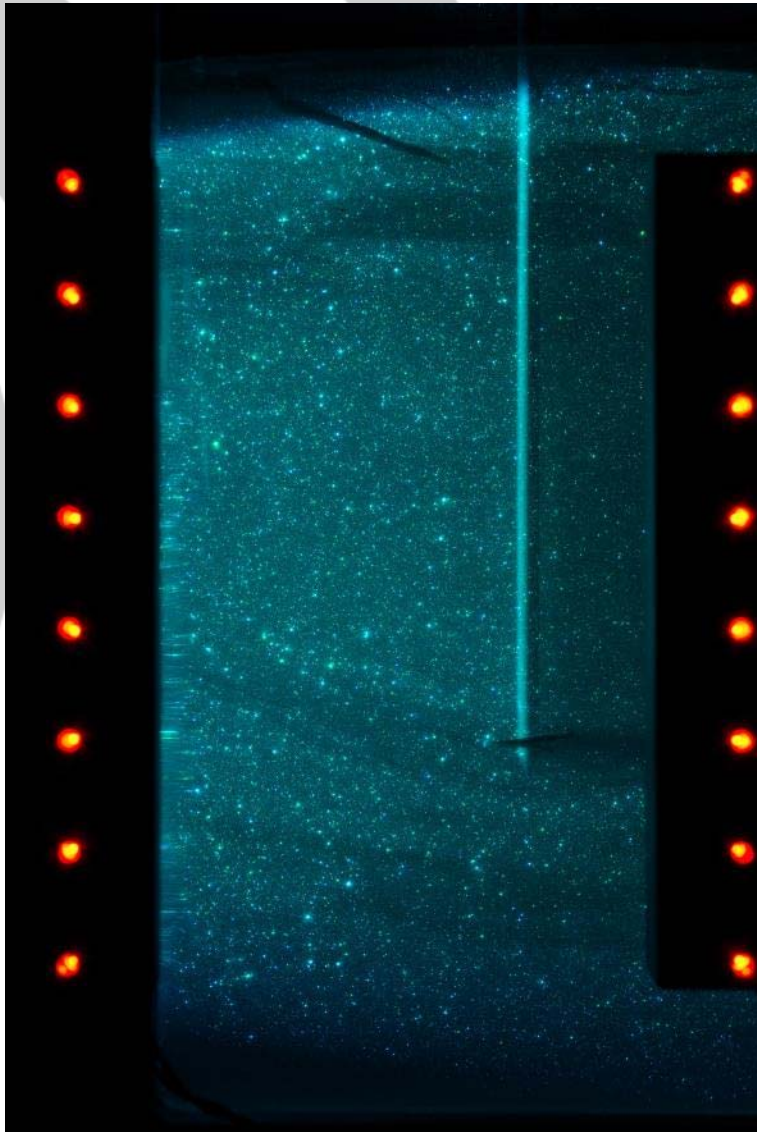




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Modelling with Transparent Soil

[Helical screw piles: test video](#)





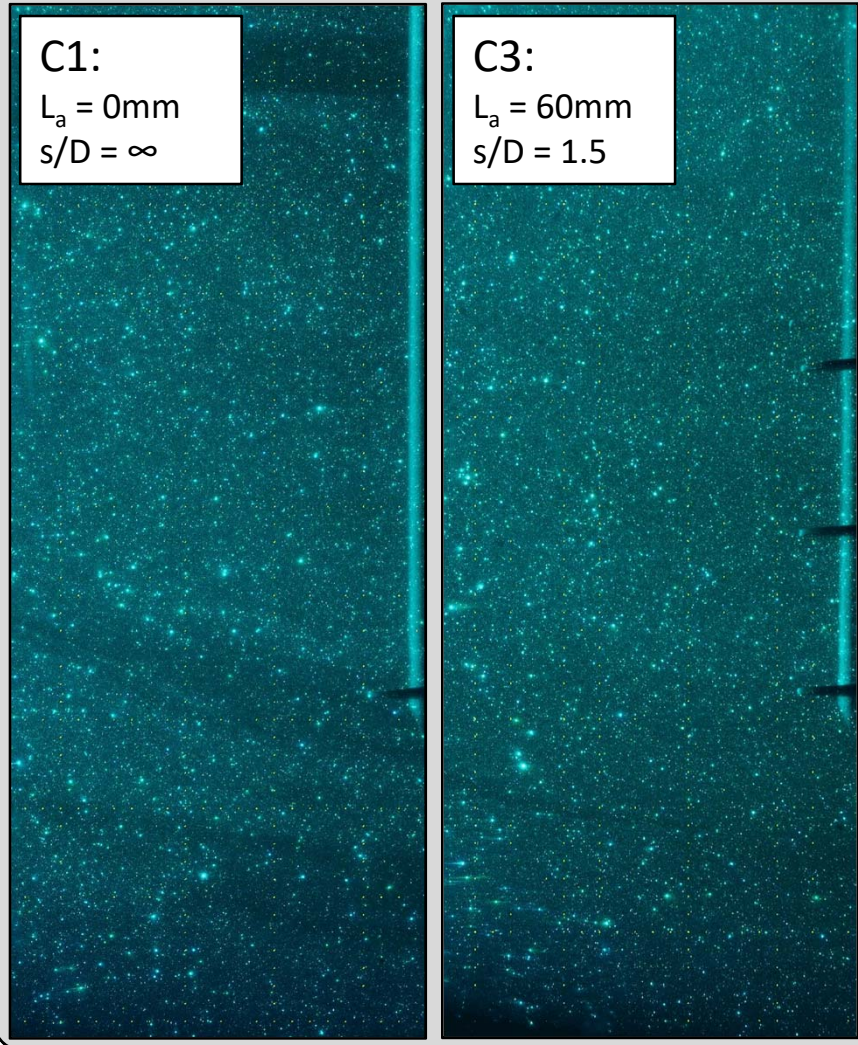
Compressive Loading

C1:

$L_a = 0\text{mm}$
 $s/D = \infty$

C3:

$L_a = 60\text{mm}$
 $s/D = 1.5$



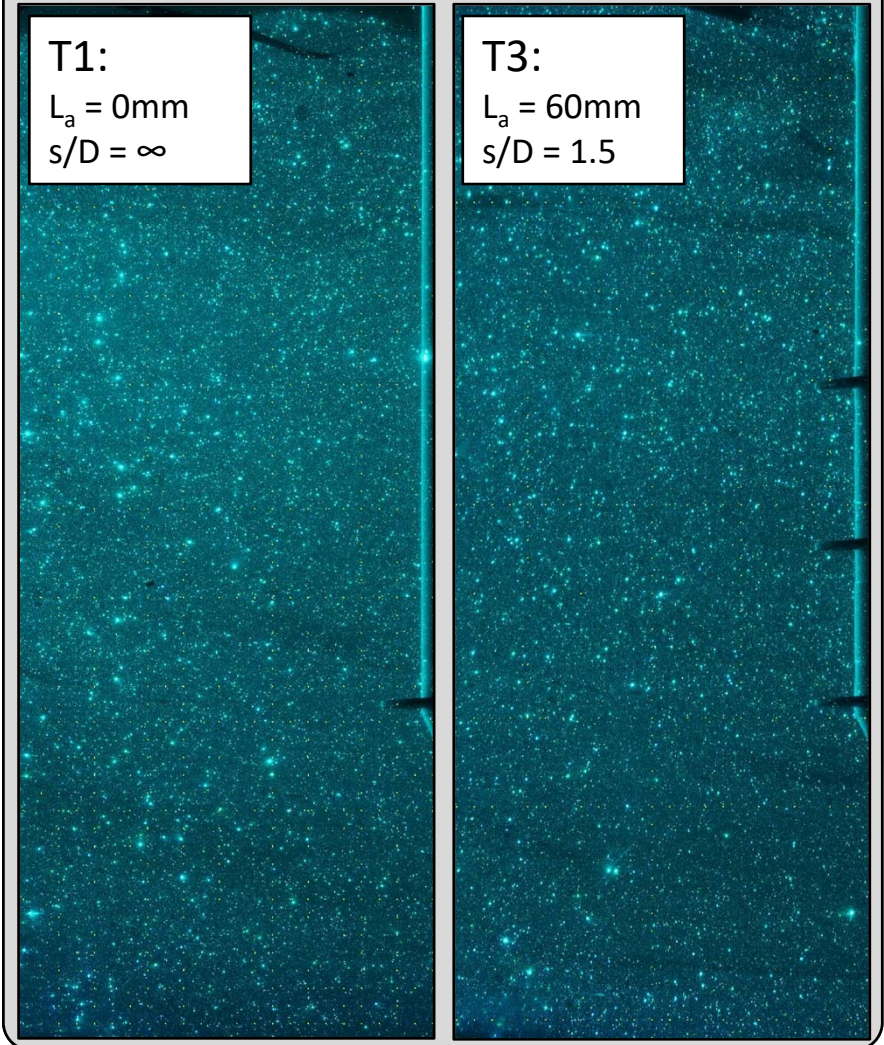
Tensile Loading

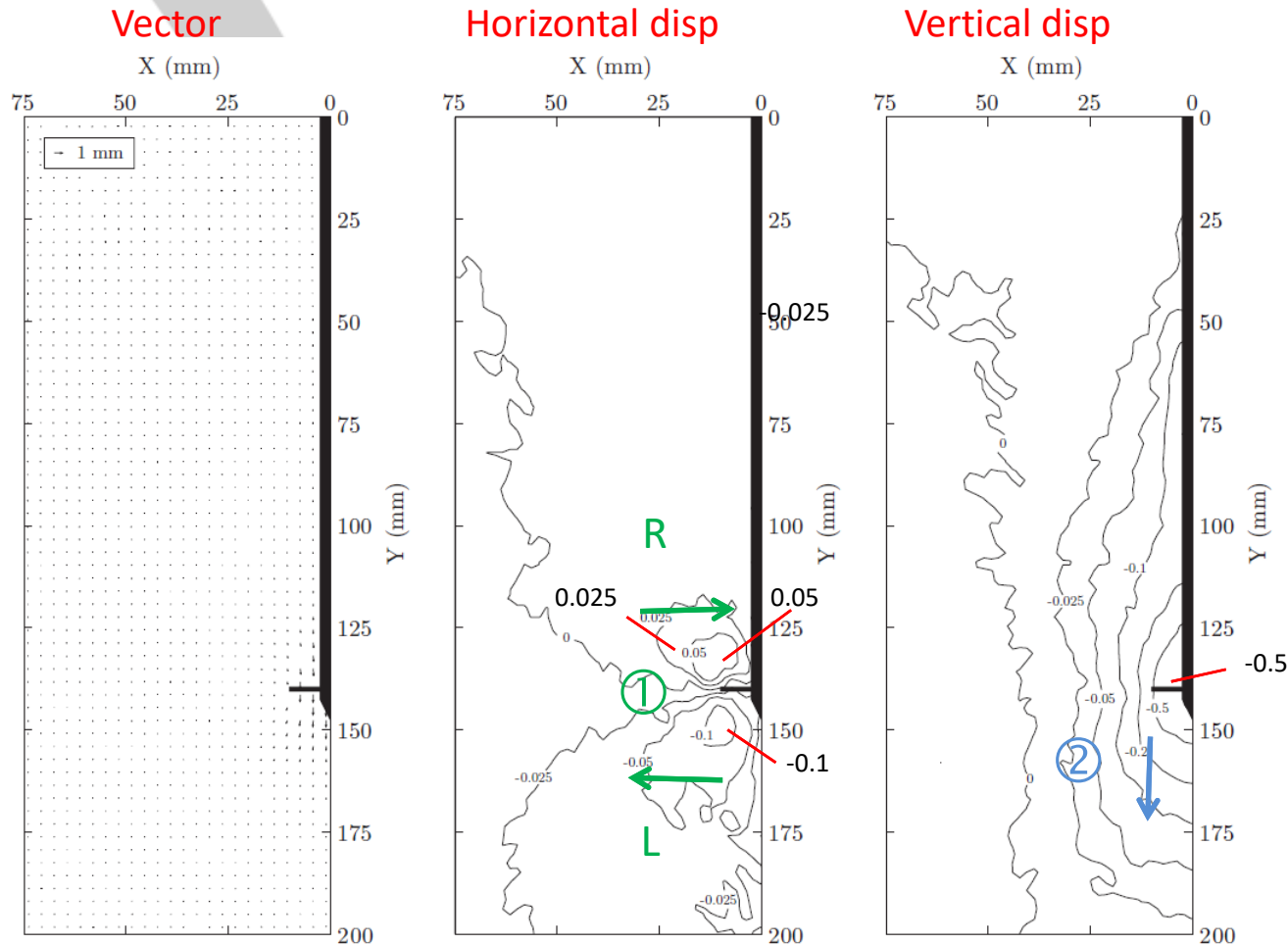
T1:

$L_a = 0\text{mm}$
 $s/D = \infty$

T3:

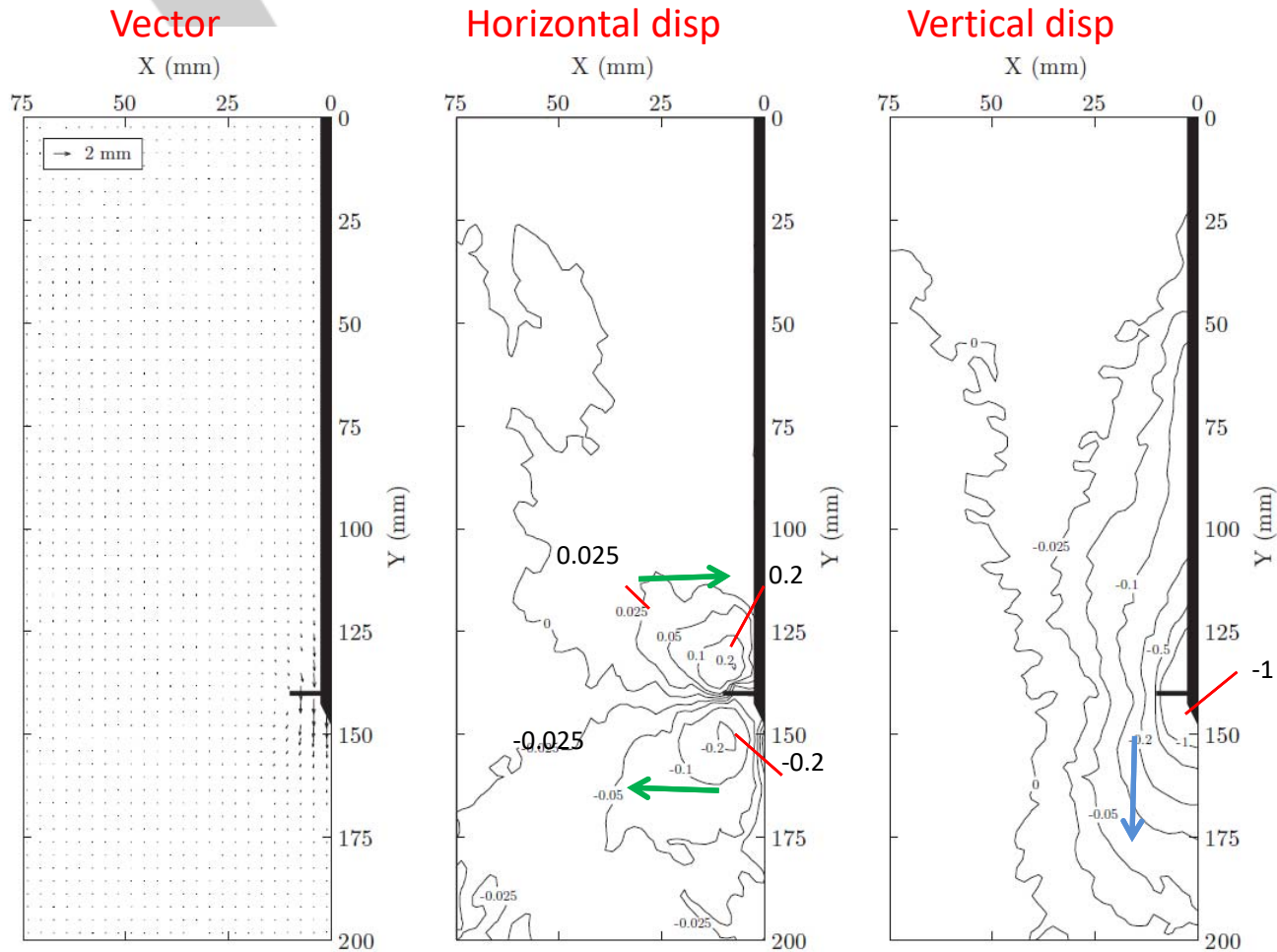
$L_a = 60\text{mm}$
 $s/D = 1.5$





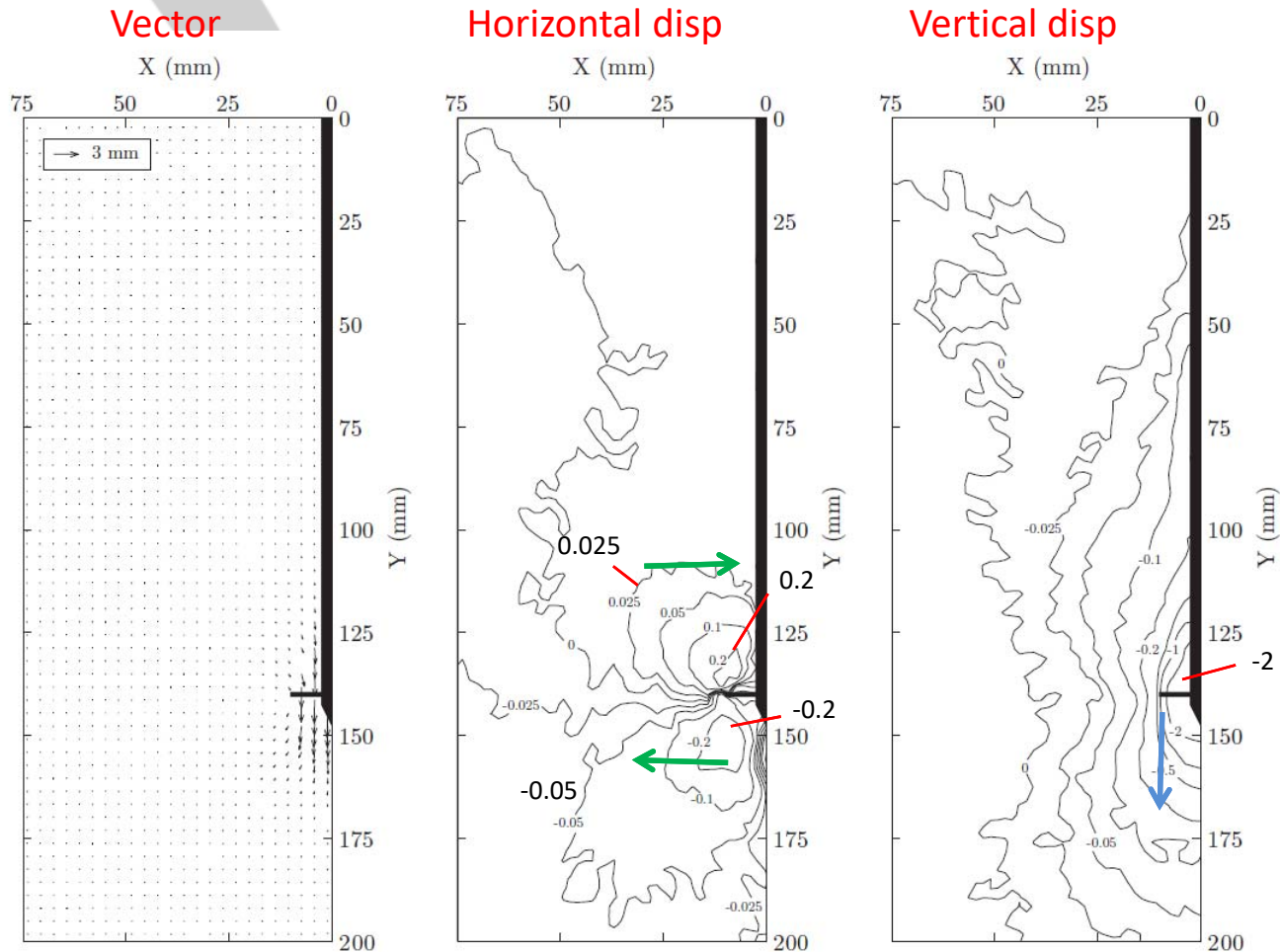
- ① Soil flow (rotation) around the plate
- ② Soil downward movement





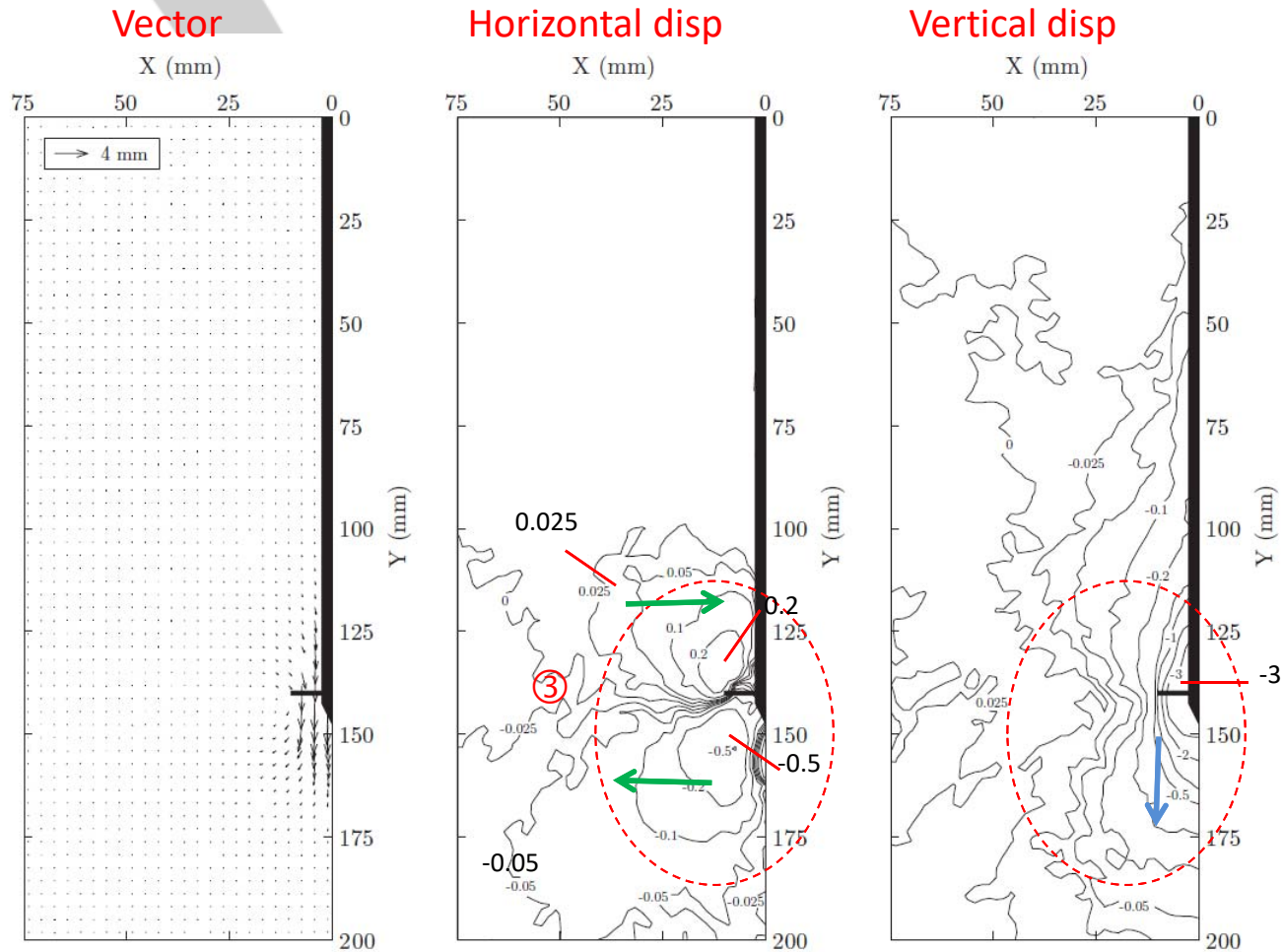
- ① Soil flow (rotation) around the plate
- ② Soil downward movement





- ① Soil flow (rotation) around the plate
- ② Soil downward movement



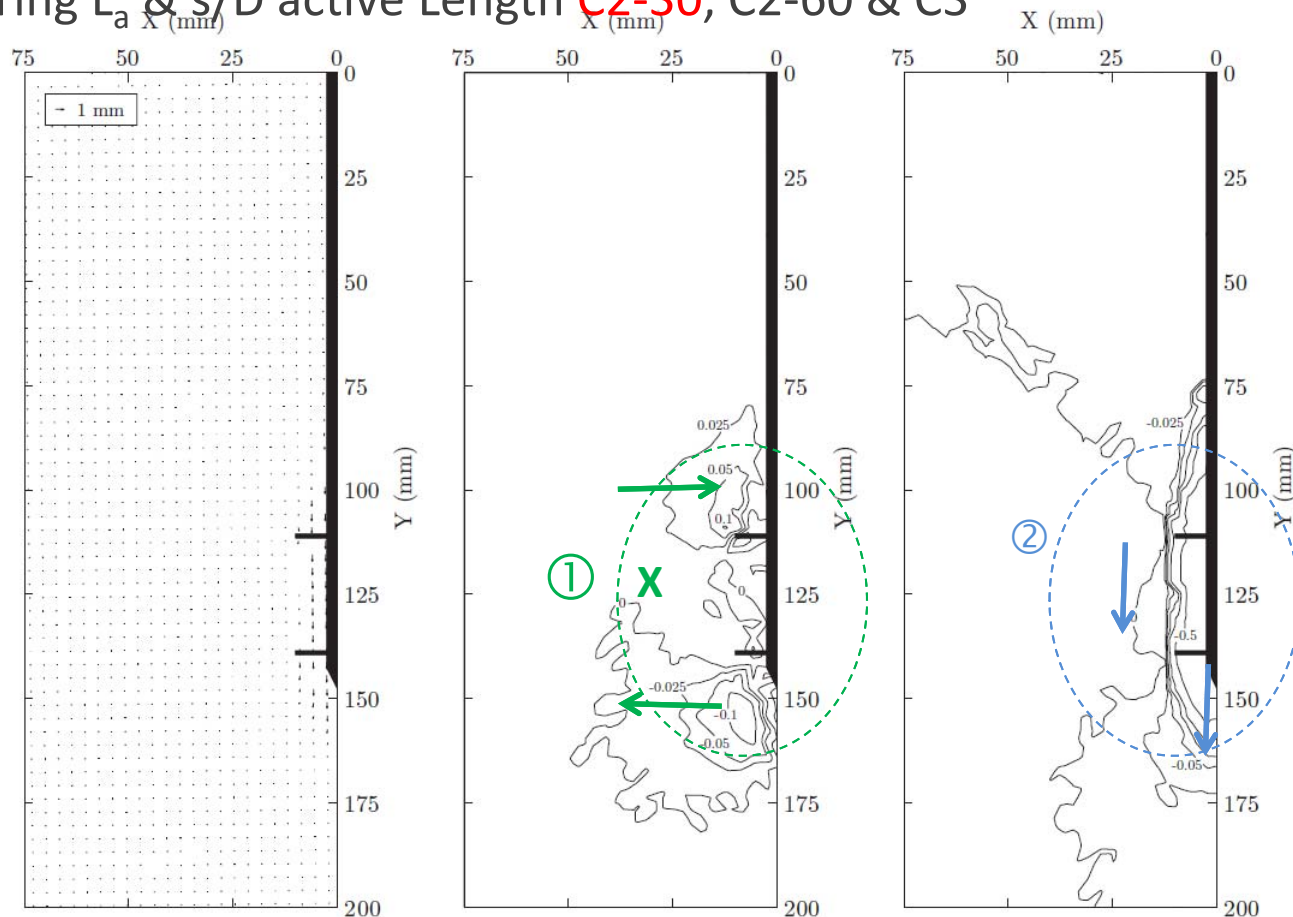


- ① Soil flow (rotation) around the plate
- ② Soil downward movement
- ③ Increased magnitudes of displacement





- Comparing L_a & s/D active Length **C2-30**, C2-60 & C3

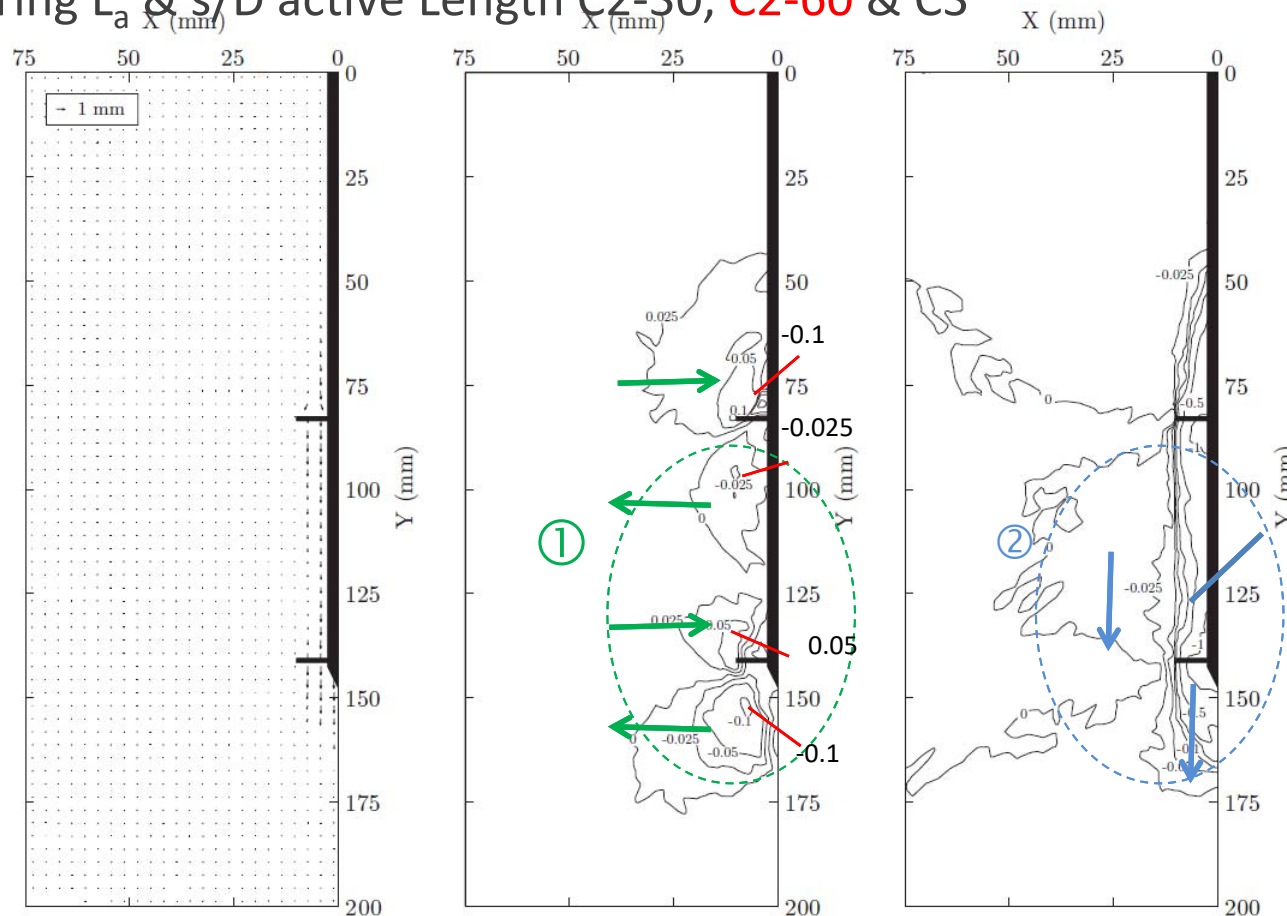


- ① Soil flow (rotation) at top/bottom plate only – None in between the plates
- ② Soil downward movement beneath – ALONG side of plates





- Comparing L_a & s/D active Length C2-30, C2-60 & C3



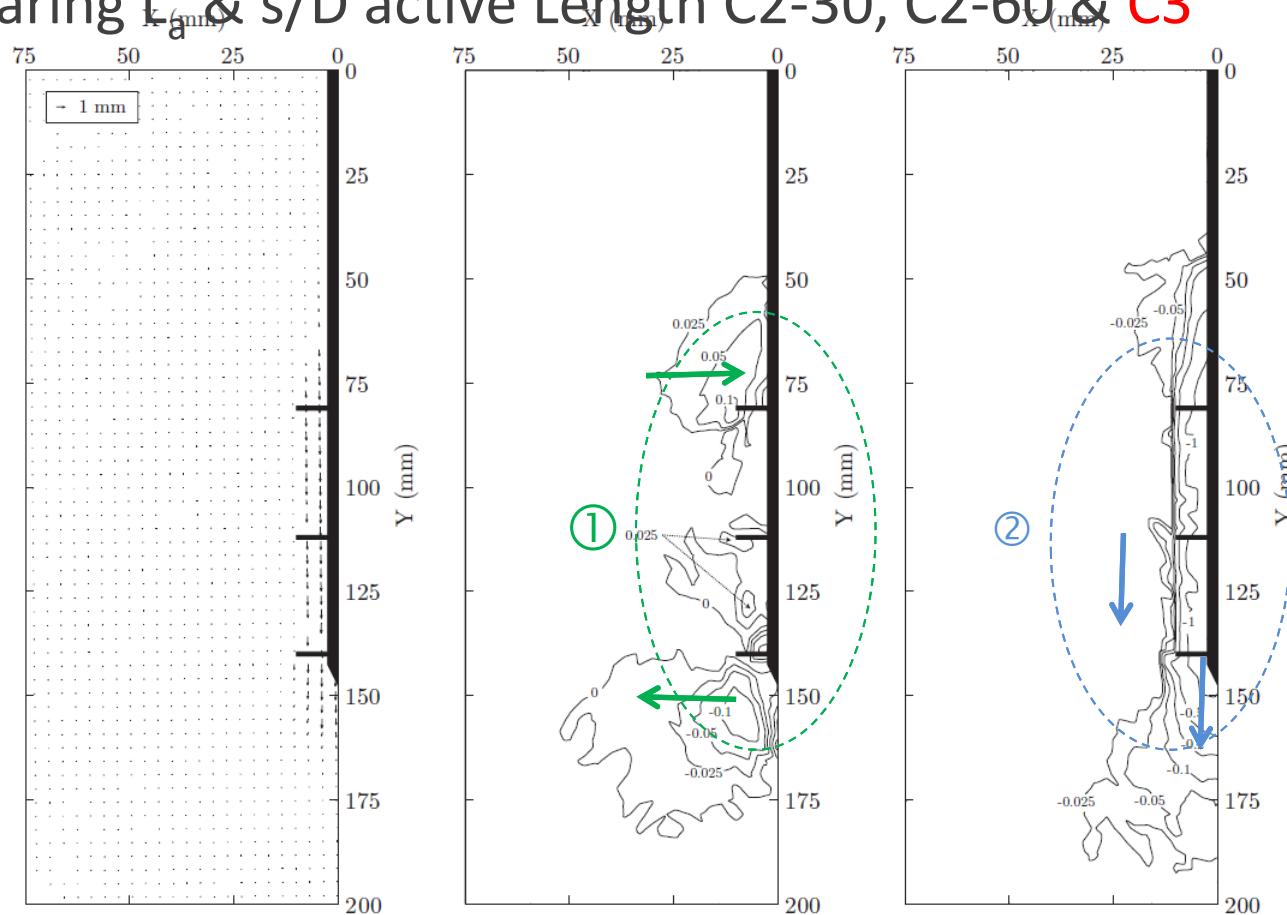
① Soil flow (rotation) around top/bottom plate & in between: Plates acting independently as spacing ratio high

② Soil downward movement beneath – ALONG side of plates but not as concentrated at higher s/D





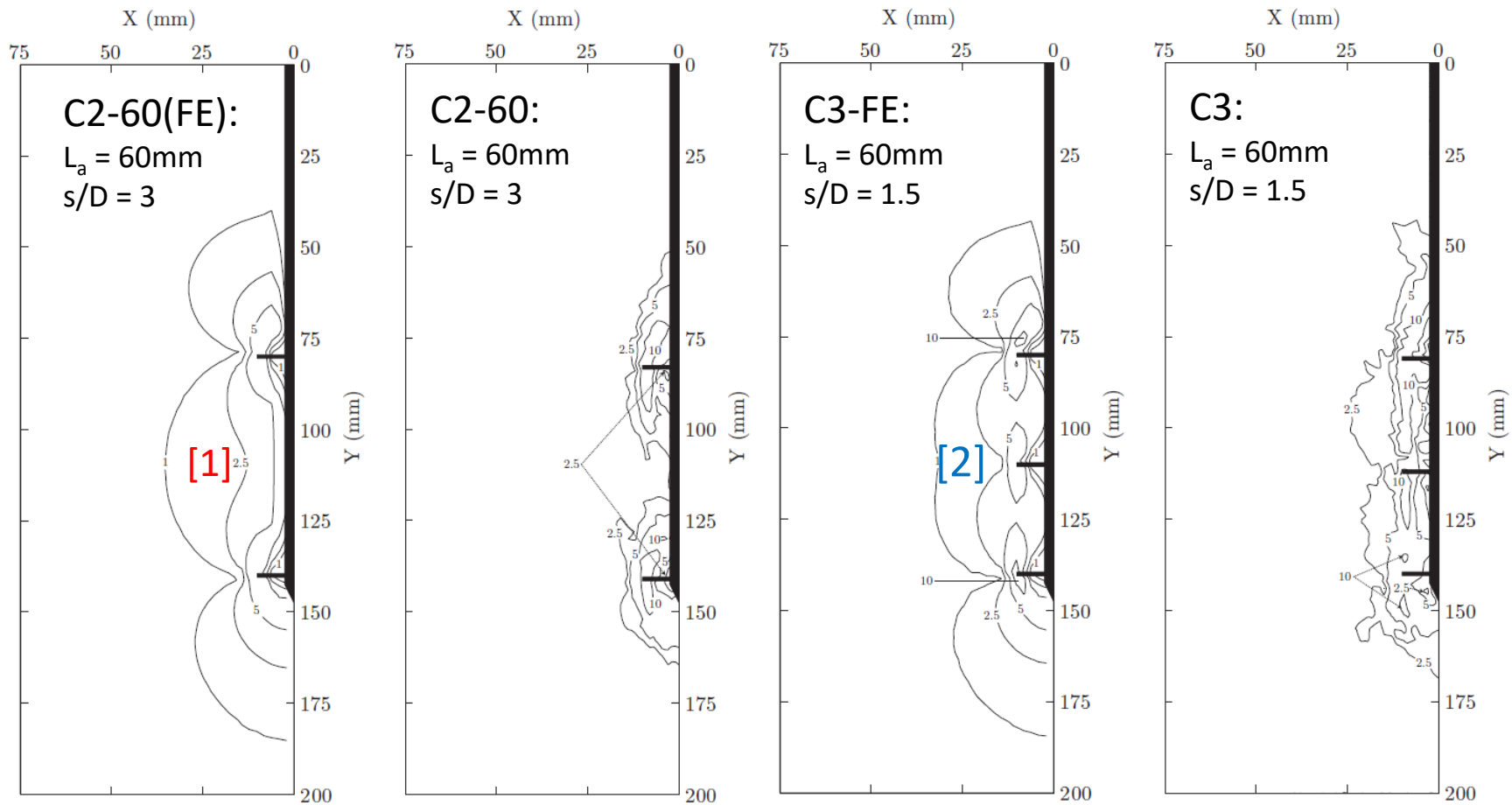
- Comparing L_a & s/D active Length C2-30, C2-60 & C3



① Soil flow (rotation) around top/bottom plate & not significantly between: soil cylinder formed

② Soil downward movement beneath – ALONG side of plates





- Shear strain observations similar
 - Isolated plates [1], shear band joining along edge [2]



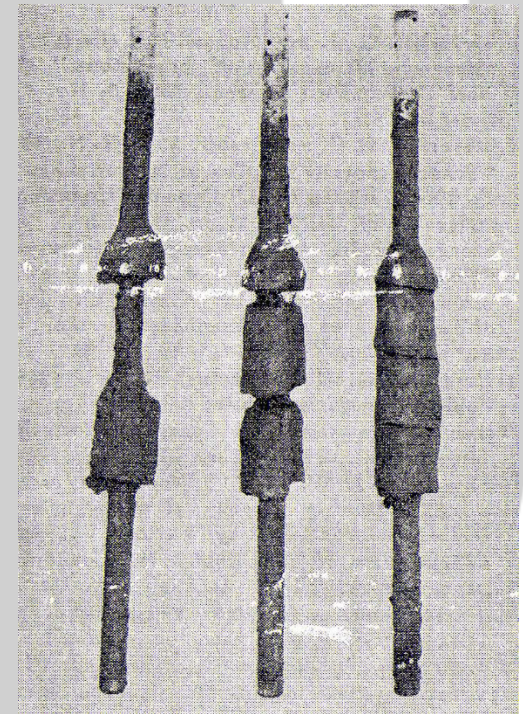


1. Failure mechanics:

- Cylindrical failure surface evident for screw piles as hypothesized by Rao et al. 1991.
- Distinct shear band seen at outer radii of helical plates
- Individual plate bearing vs cylindrical failure dependent on s/D ratio

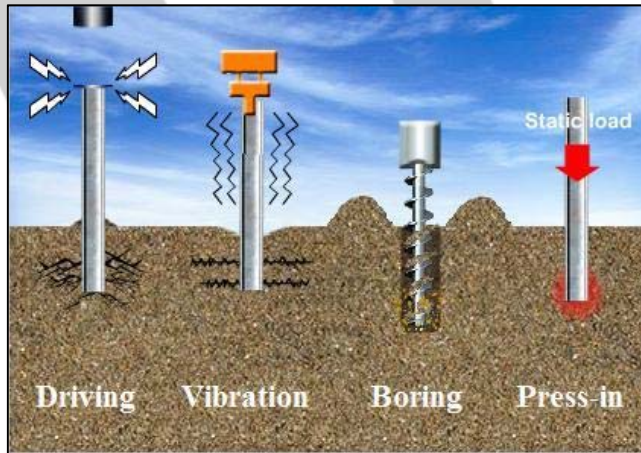
2. Design implications:

- Correct selection of failure mode
- ULS capacity governed by L_a
- Rate of capacity gain governed by s/D ratio





- Classical Piling methods (driving, vibration, boring)
 - vibrations and soil disturbance



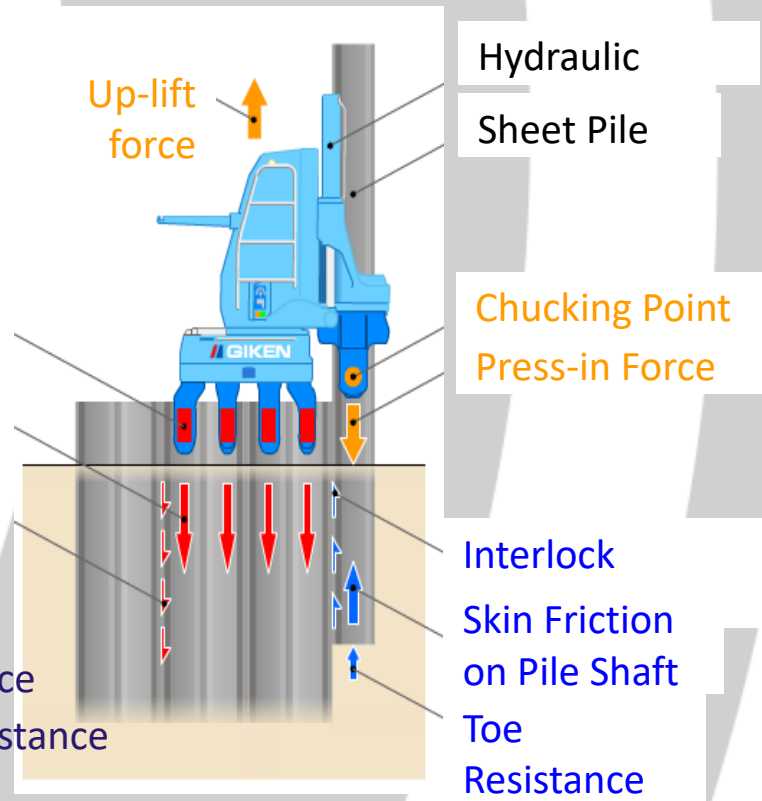
- Reduce installation force
- Ground disturbance



Clamping Points

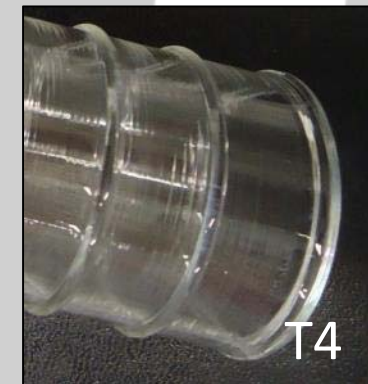
Skin Friction on Reaction Piles
Interlocking Resistance

■ Reaction Force
■ Press-in Resistance





Test	Pile configuration						
	Material	Length (mm)	Straight	Shoe	Rib	Spacing (s:mm)	D _{outer} (mm)
T1	Glass	400	X	-	-	-	46
T2	Glass	400	-	X	-	-	49
T3	Glass	400	-	X	X	30	49
T4	Glass	400	-	X	X	15	49

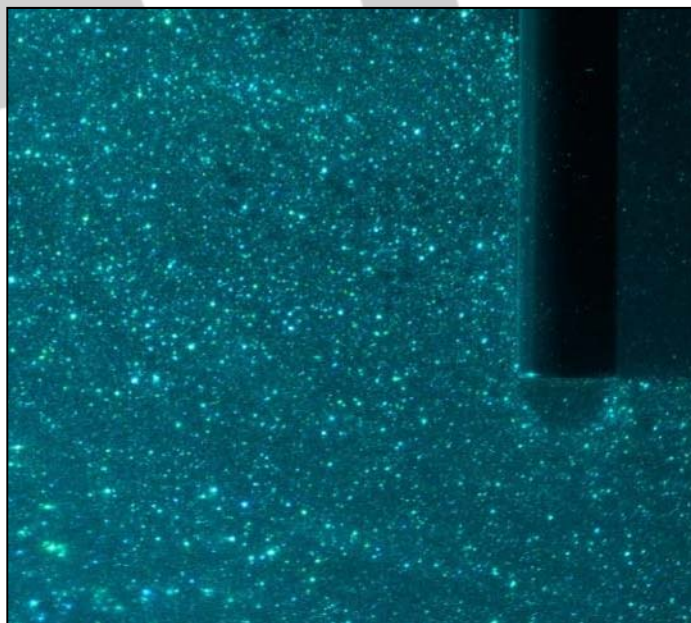


➔ Piles made from glass to allow internal visualisation.

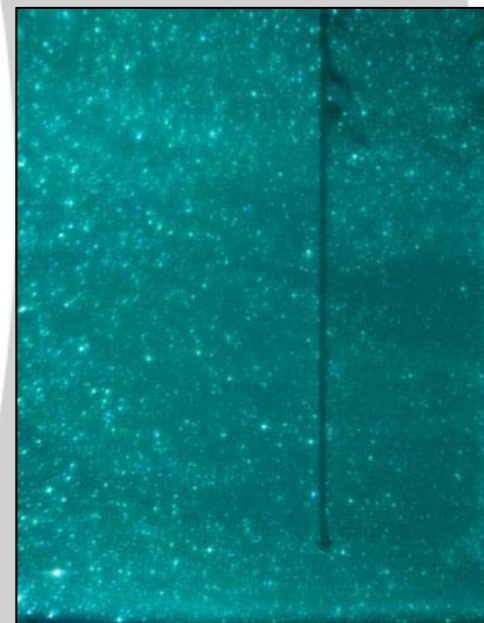




- Internal visualisation possible



Shadow



No Shadow





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Modelling with Transparent Soil

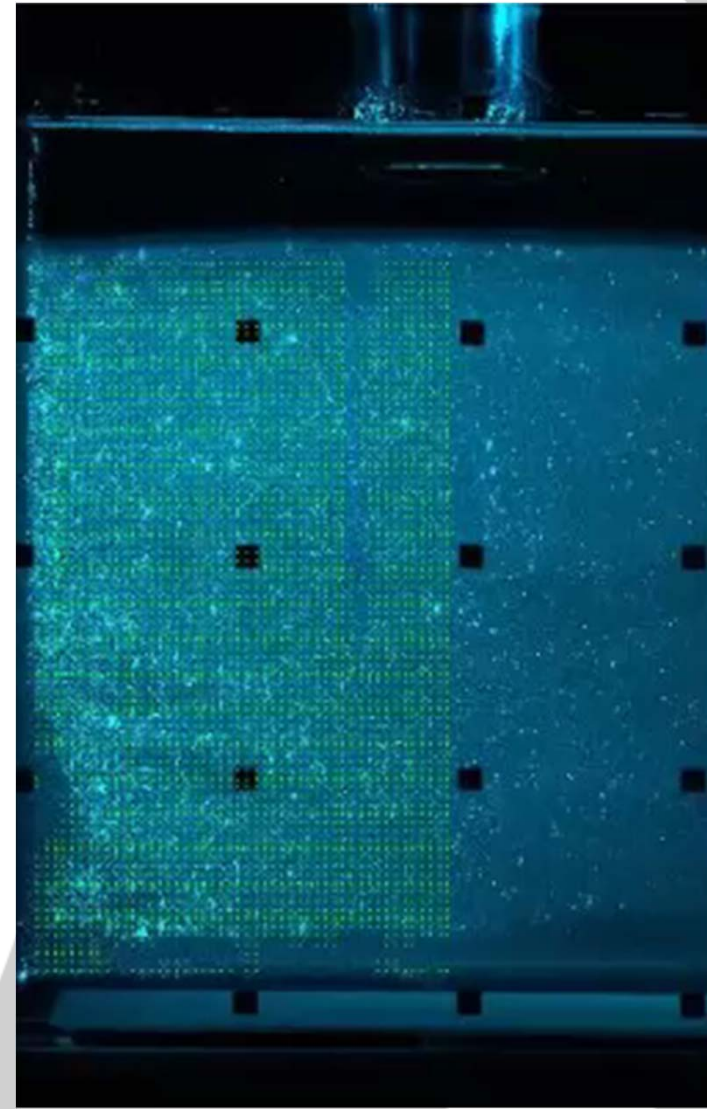
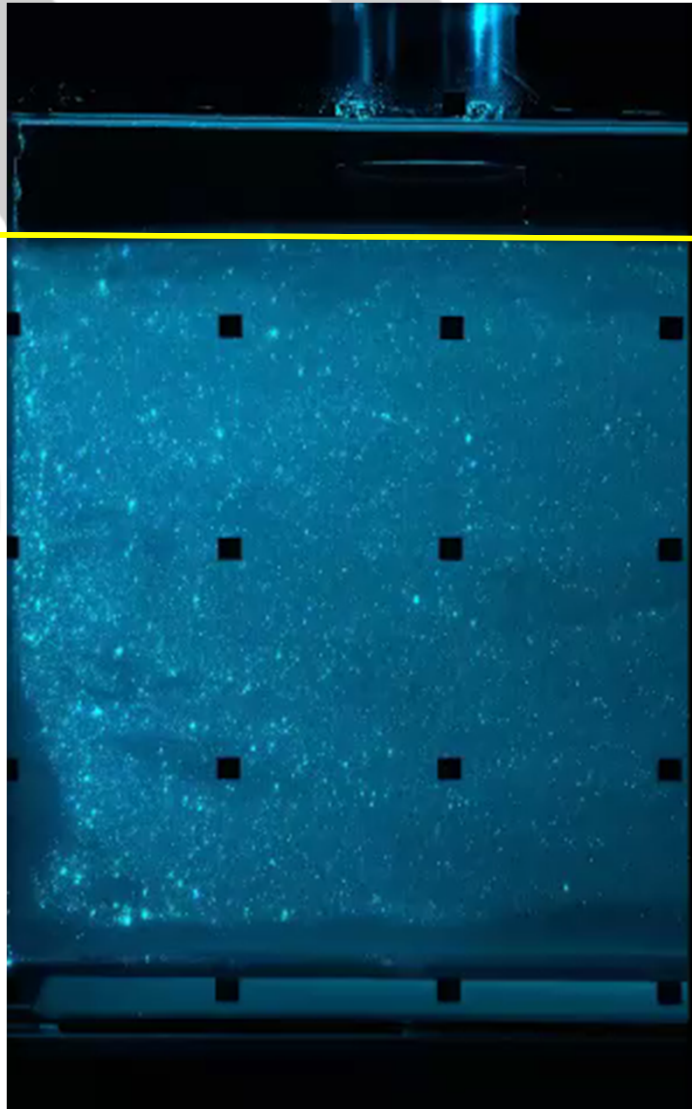
Press-in piling: internal observations

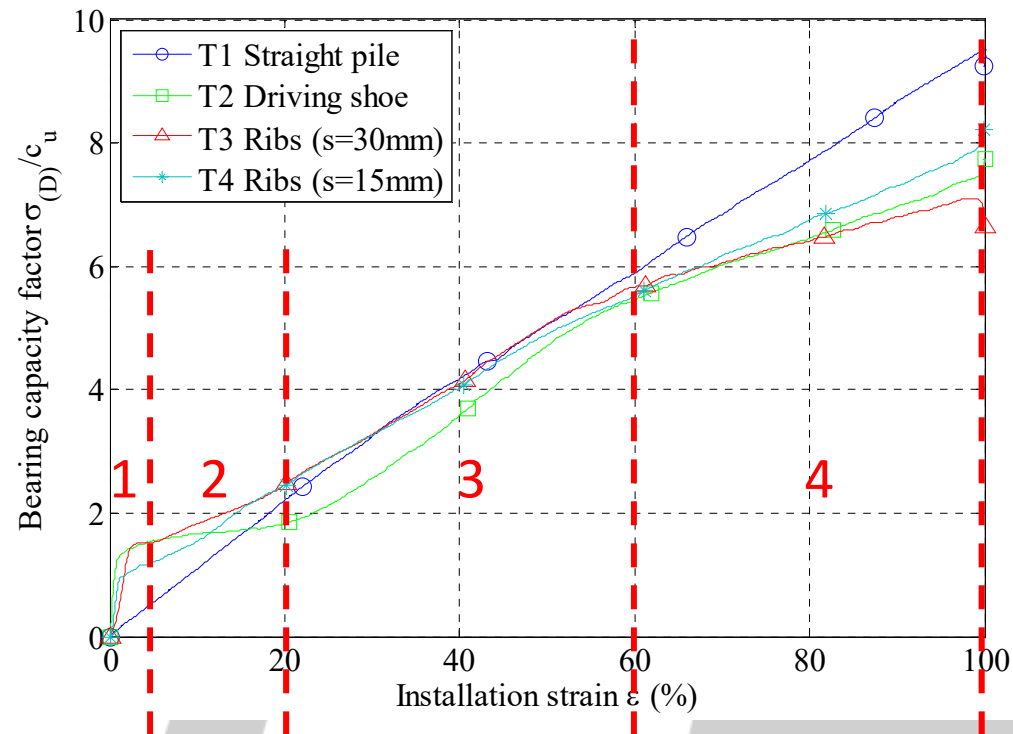




Modelling with Transparent Soil

Press-in piling: internal observations





1. Base resistance increased and influenced by the driving shoe
2. Horizontal movement away from the pile
3. Flow around condition and full contact at side
4. Ribs impact and soil plug

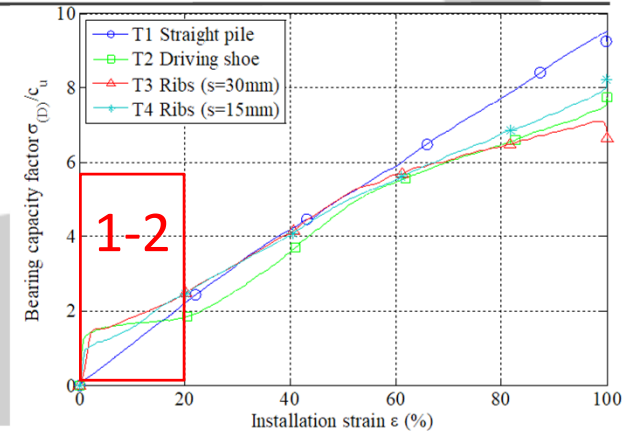




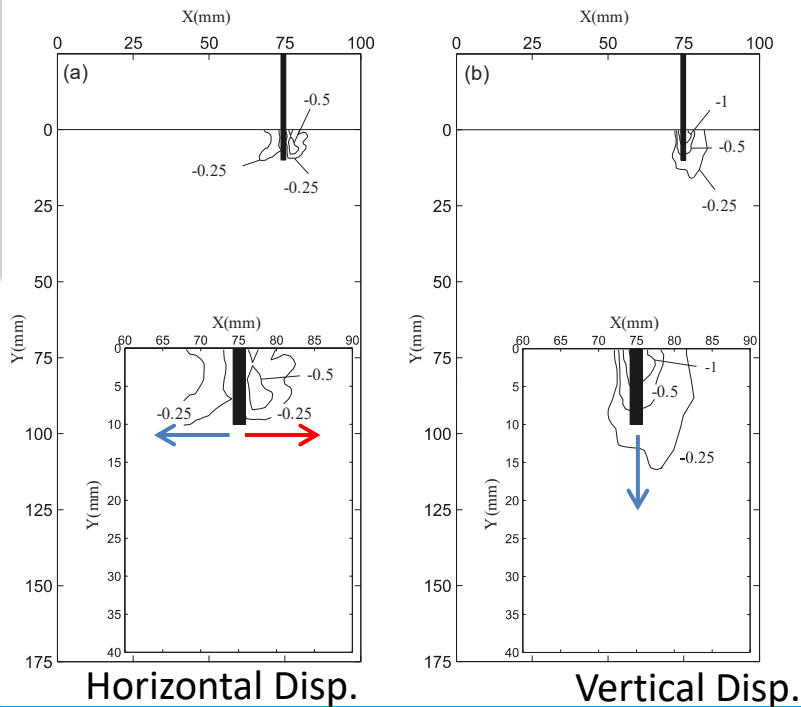
Modelling with Transparent Soil

Soil displacement: initial penetration in Zone 1 - 2

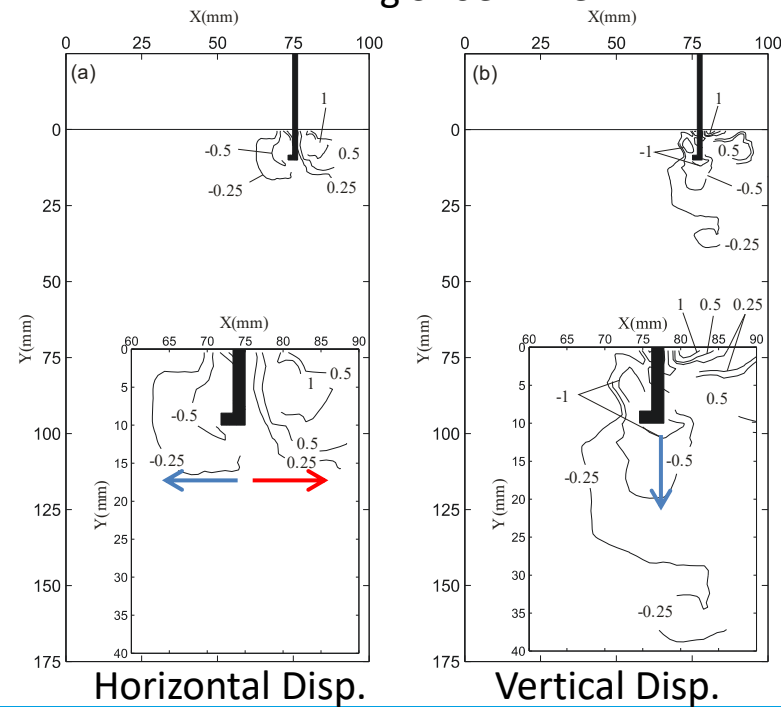
- T1: [1] resistance generated through the shaft friction
[2] no initial increase in bearing resistance
- T2, T3, T4:
[1] increase of initial resistance
[2] more soil moved due to the driving shoe



Straight Pile

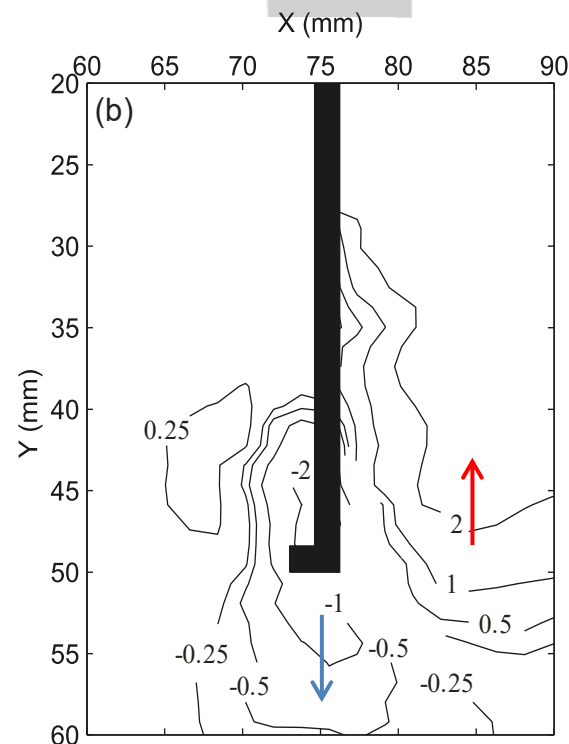
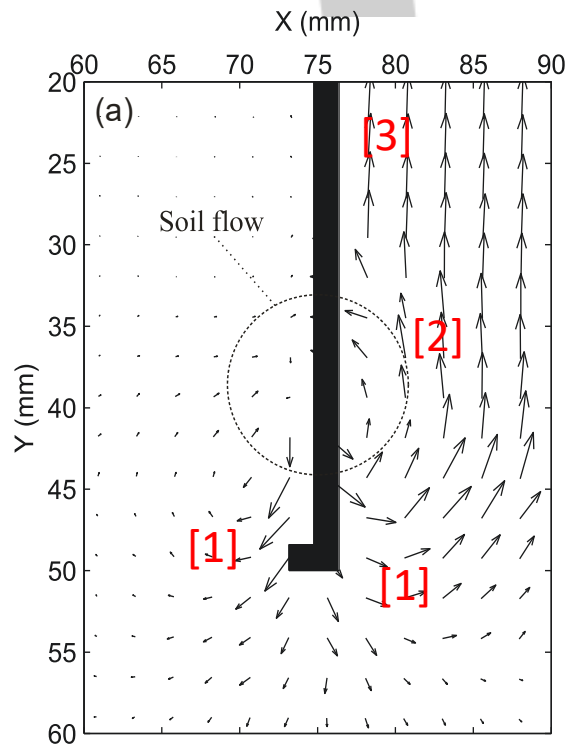
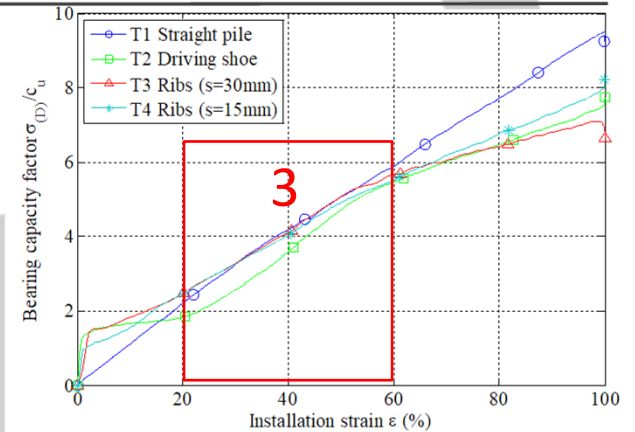


Driving Shoe - Pile





- [1] Soil movement away from the outer and inner sides of the shoe
- [2] Soil rotation back towards the side increasing confinement & the internal stress
- [3] Re-engagement of the shaft frictional resistance on load plot

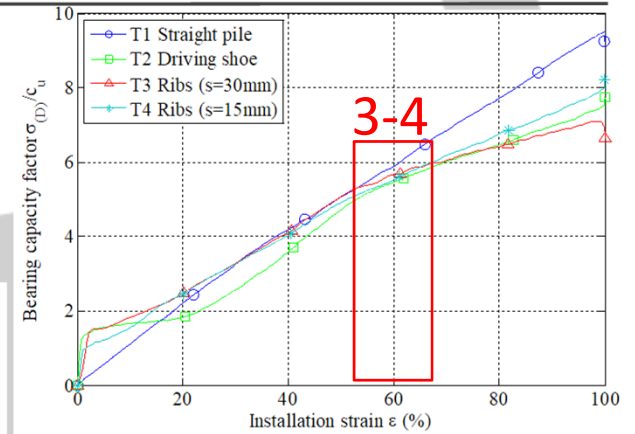
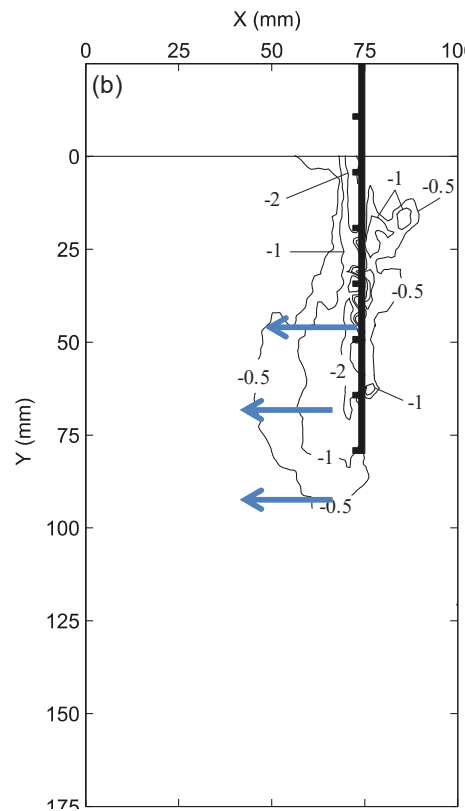
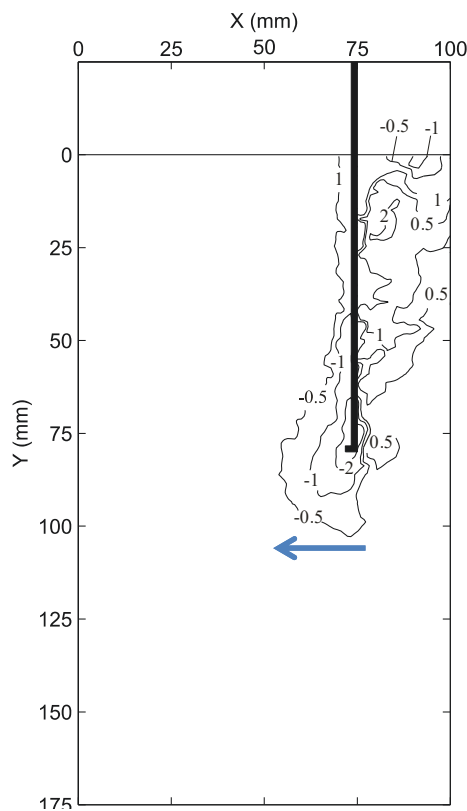




Modelling with Transparent Soil

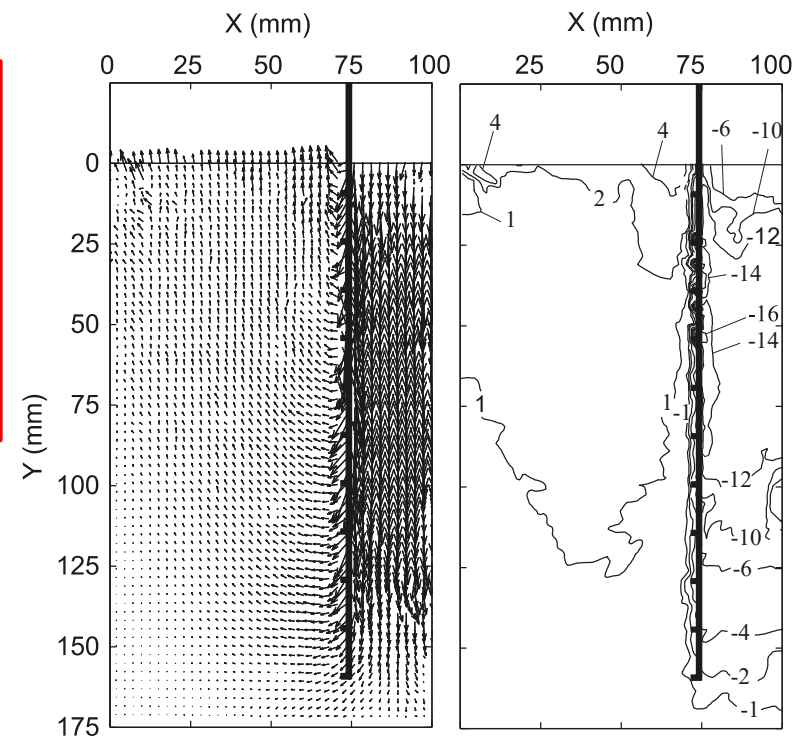
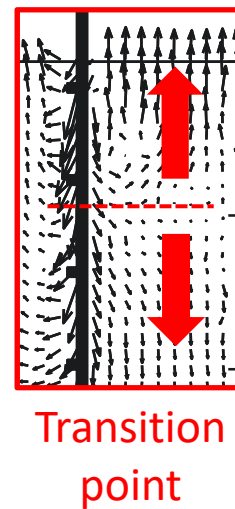
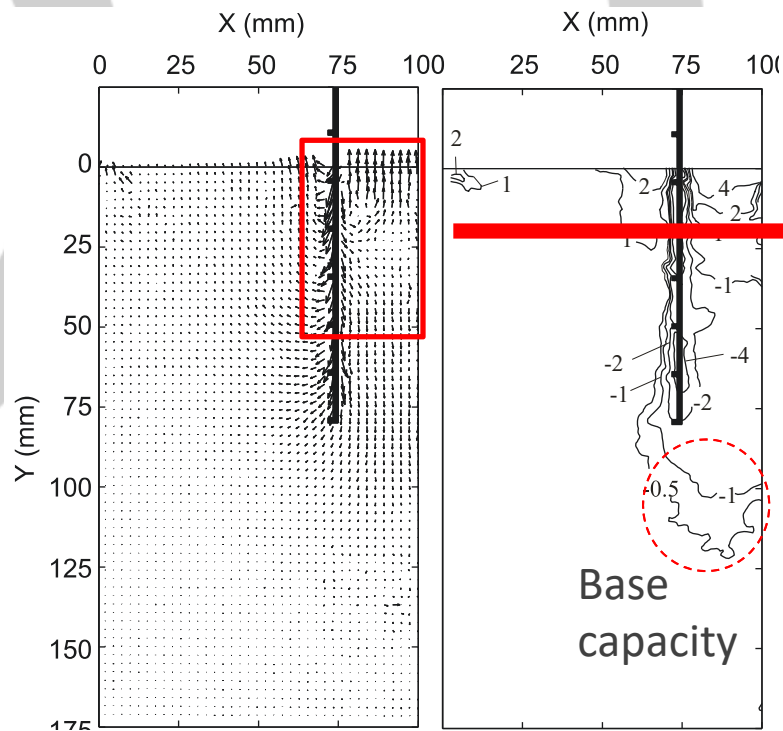
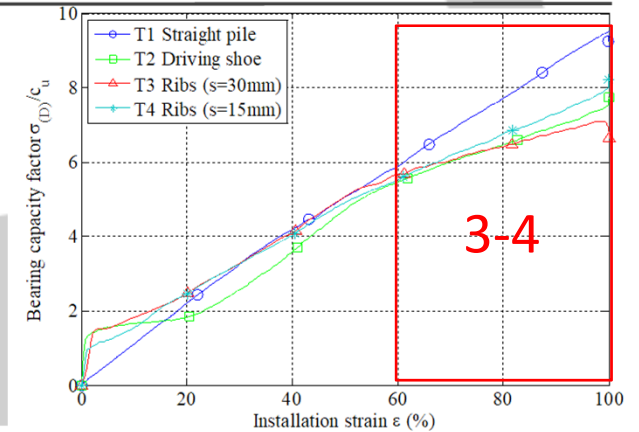
Soil displacement: impact of surface body ribs

- Increased horizontal movement away from pile side along the length

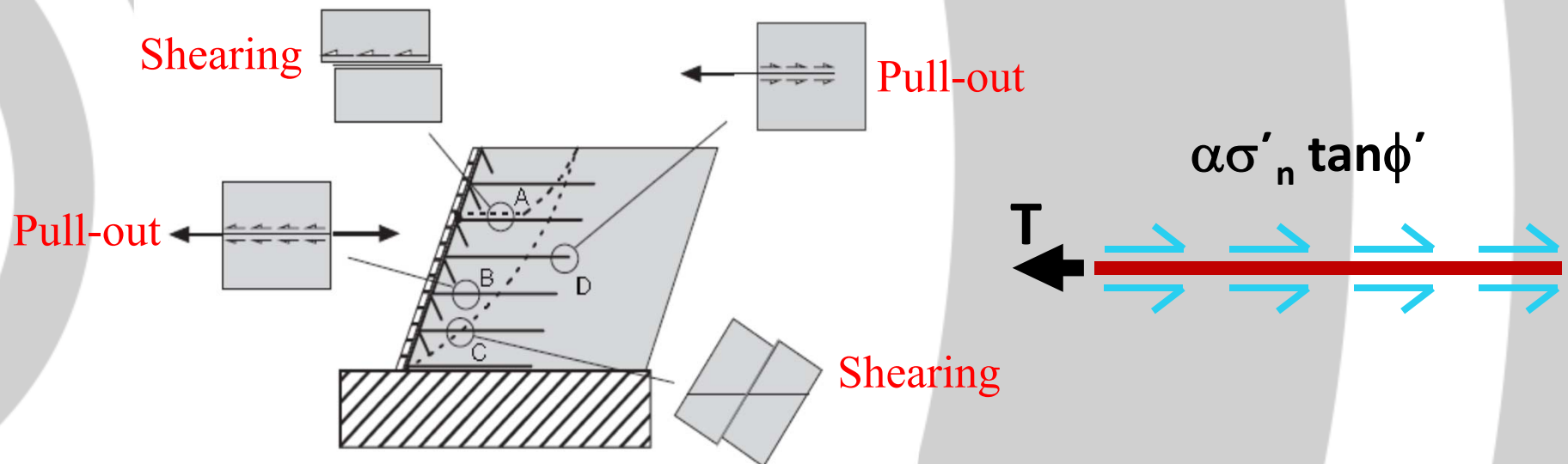




- Soil plugging
 - change in soil direction inside the pile;
 - onset of soil plugging inside the pile where a lost in shaft friction occurs
 - 'Switch on' of base resistance



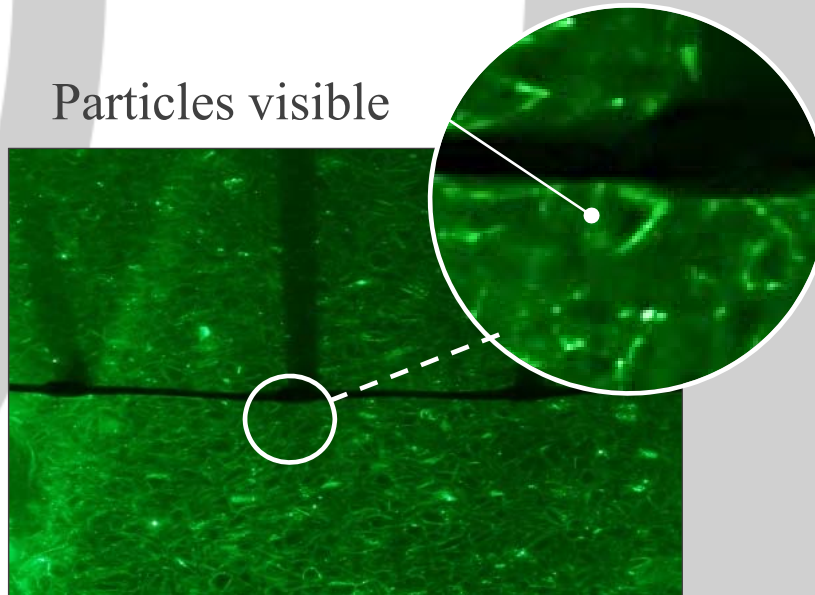
Interaction in geo-reinforcement presents a major challenge in design



- Interface coefficient (α) not well understood and defined in a broad range in design [$\alpha = 0.6$ to 0.8]
- Need for greater understanding of interaction mechanics



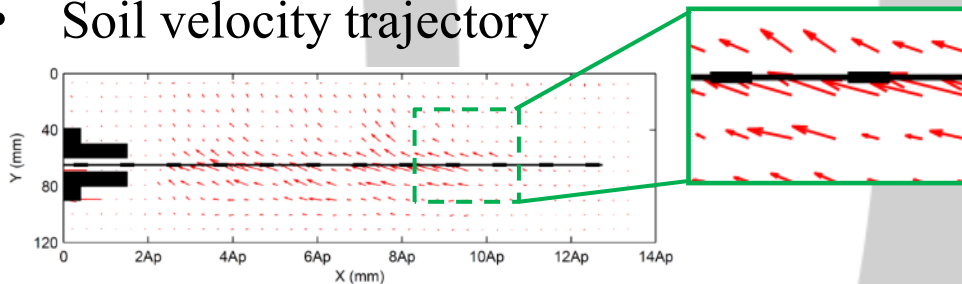
Particles visible



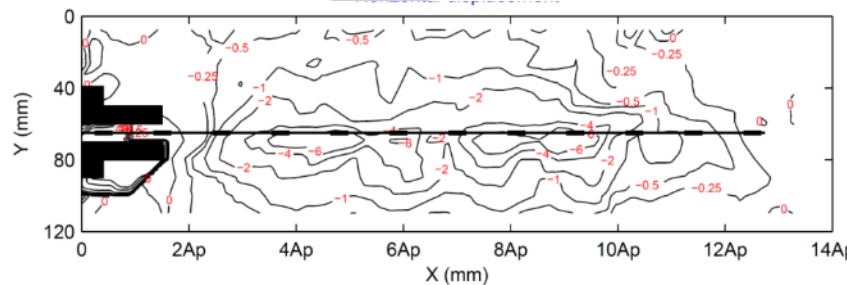


Data output and appraisal

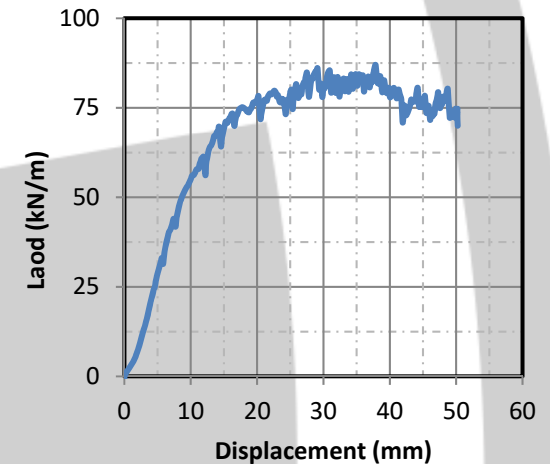
- External – Load / Disp. behaviour
- Internal – Soil interaction behaviour [DIC]
 - Soil velocity trajectory

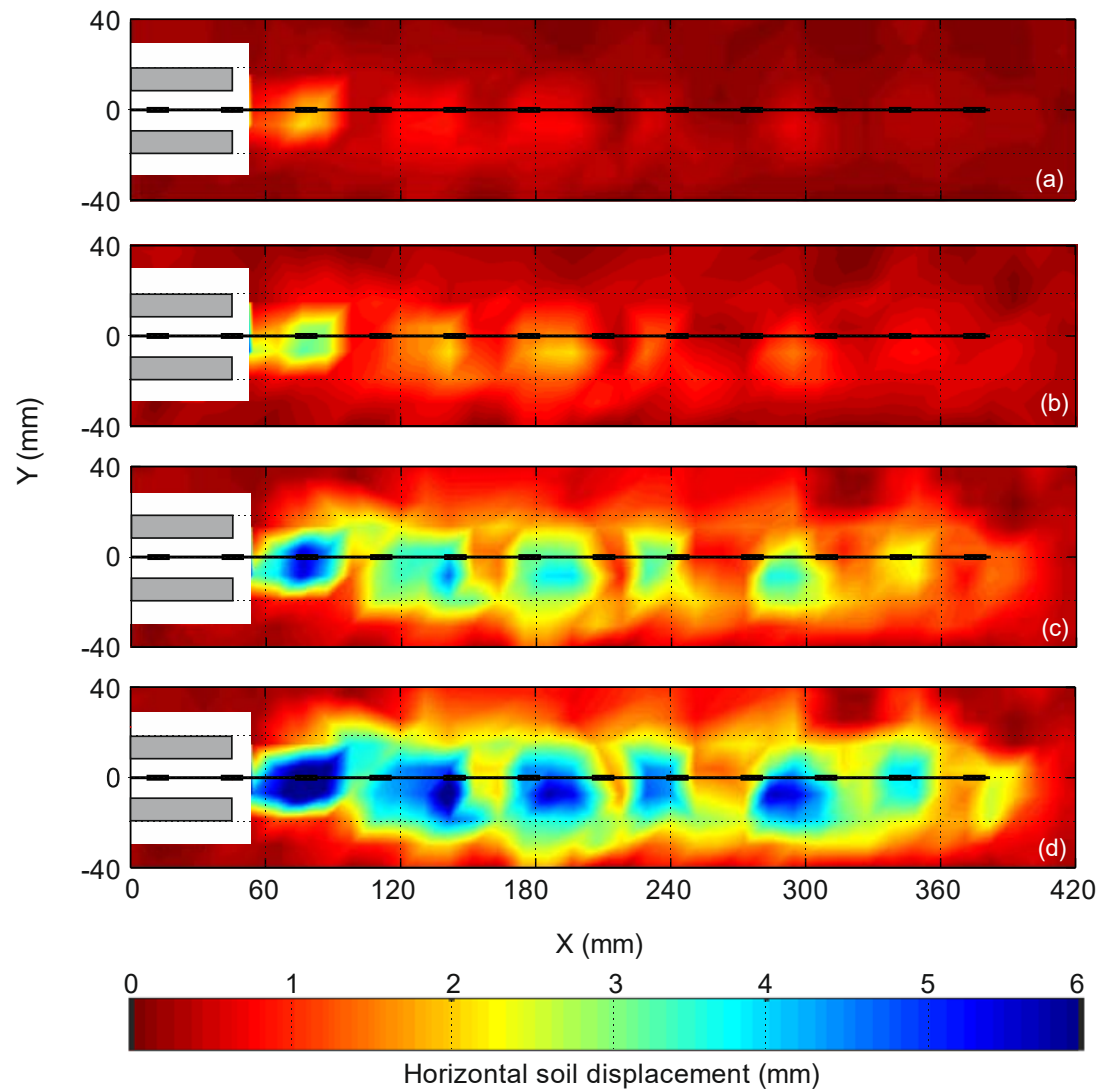


- Vertical & Horizontal Contours of displacement



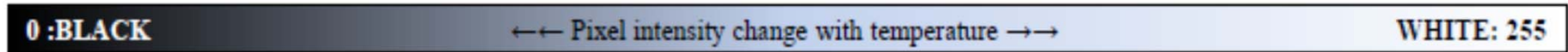
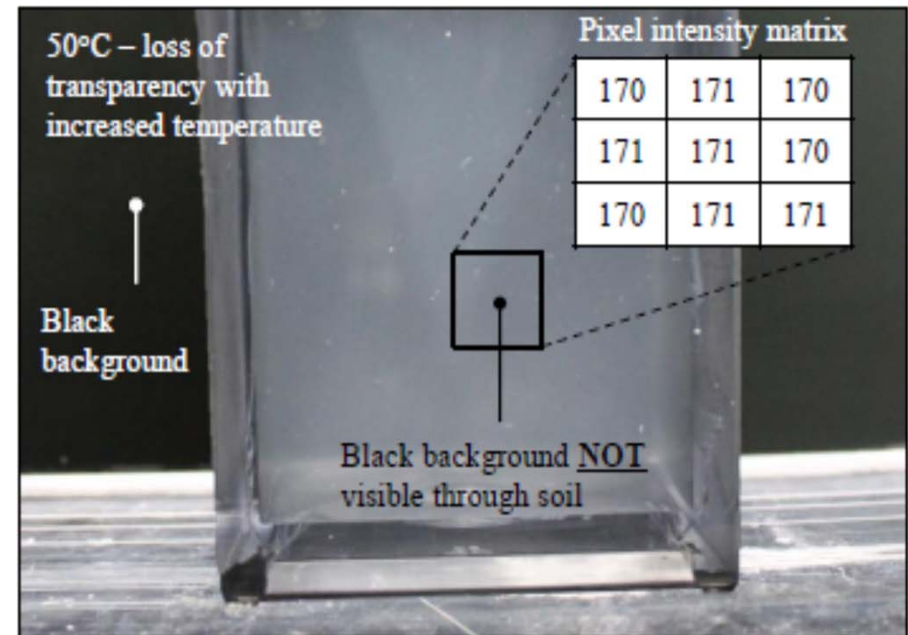
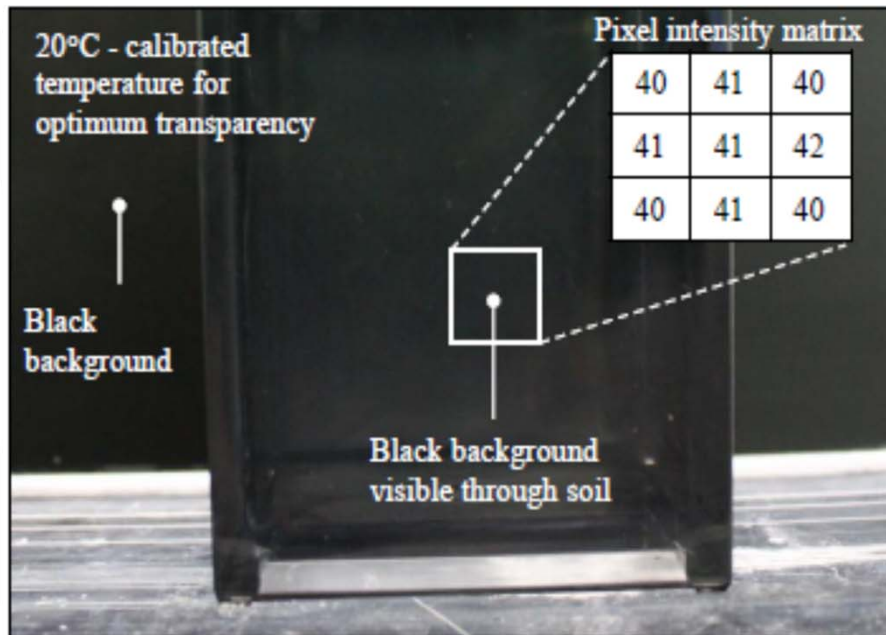
- Region of soil movement
- Zone of influence
- Local & global response (cumulatively & incrementally)

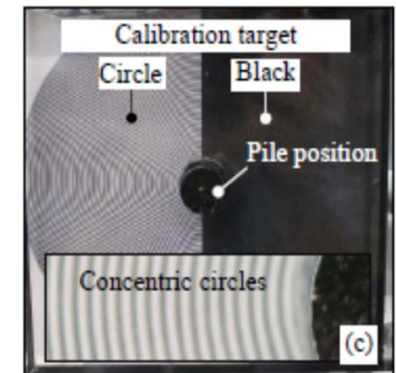
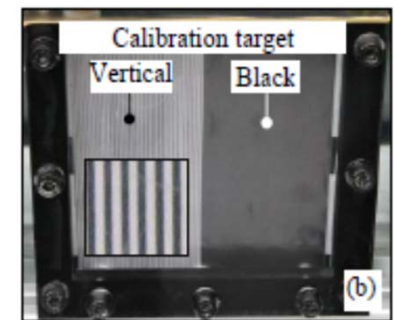
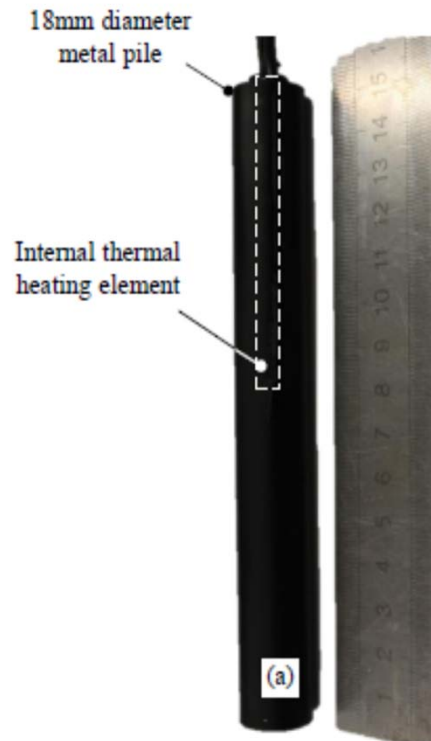
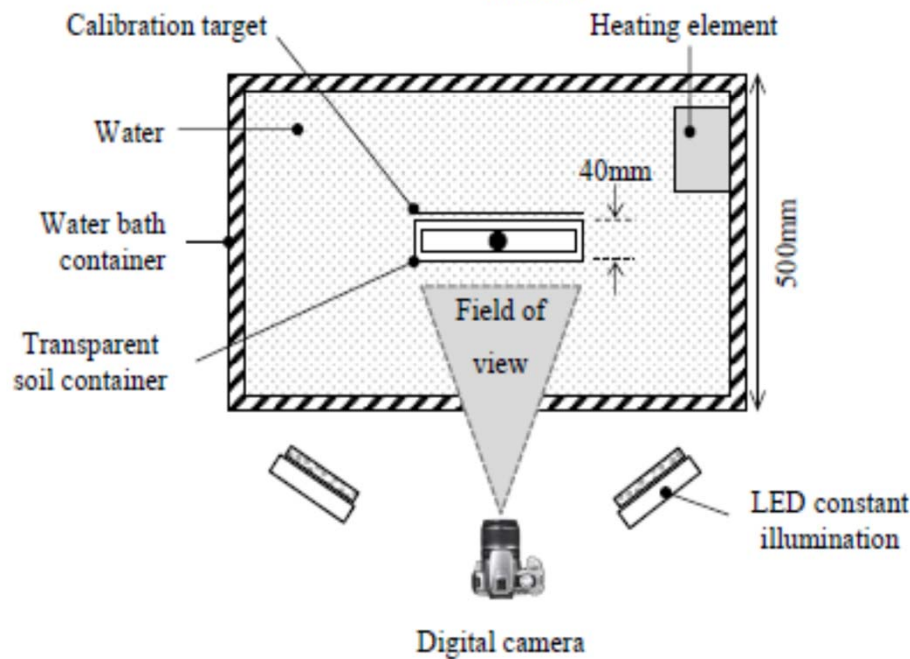
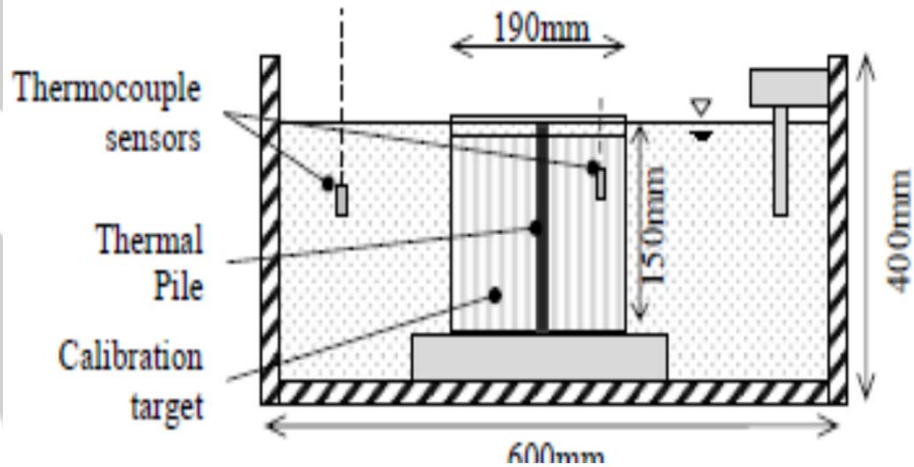






Energy Piles



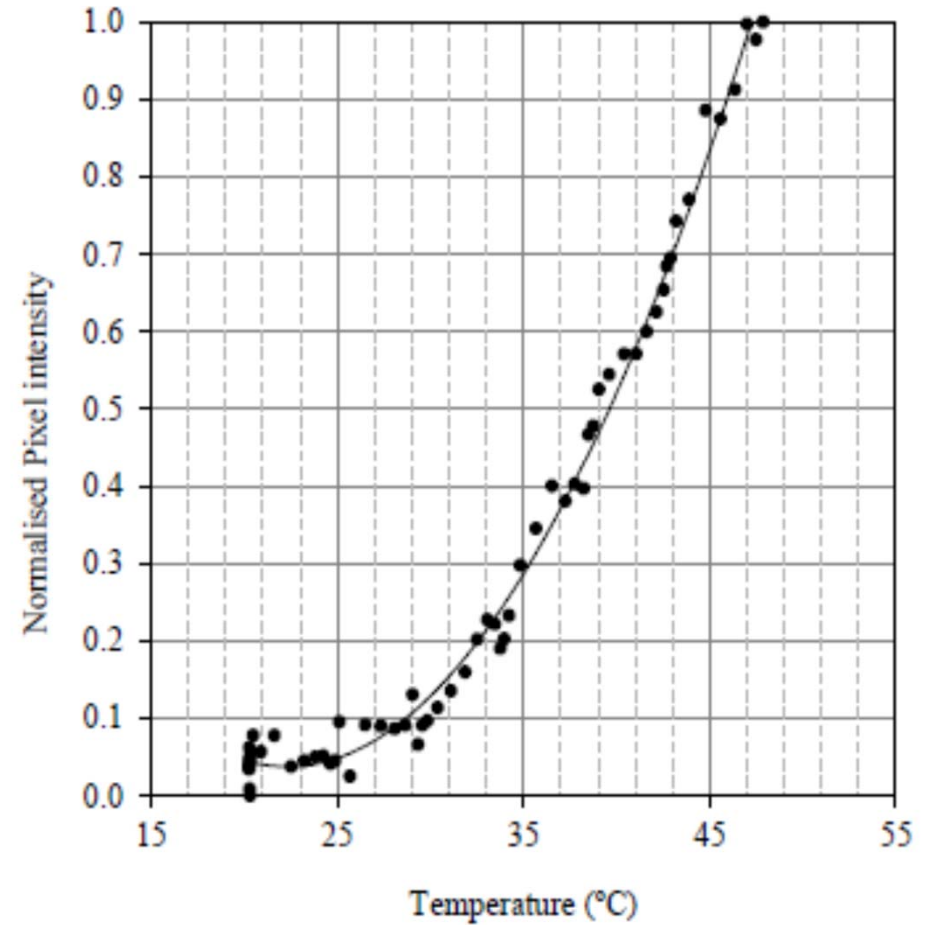
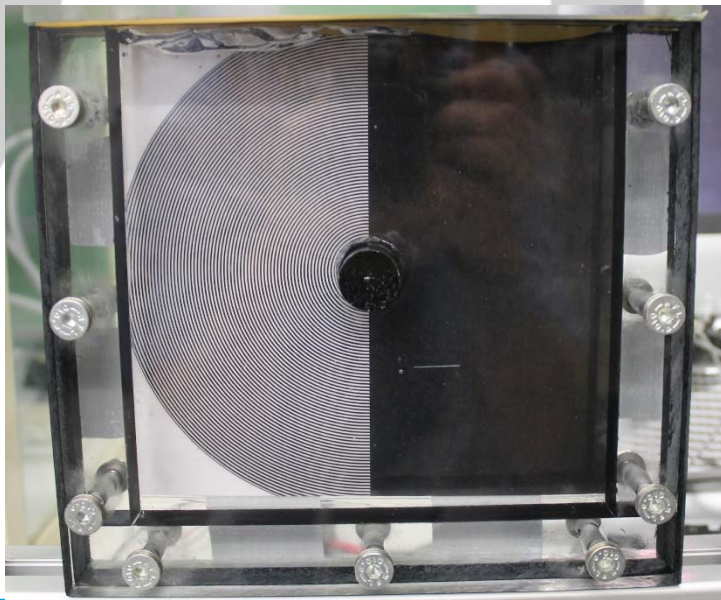
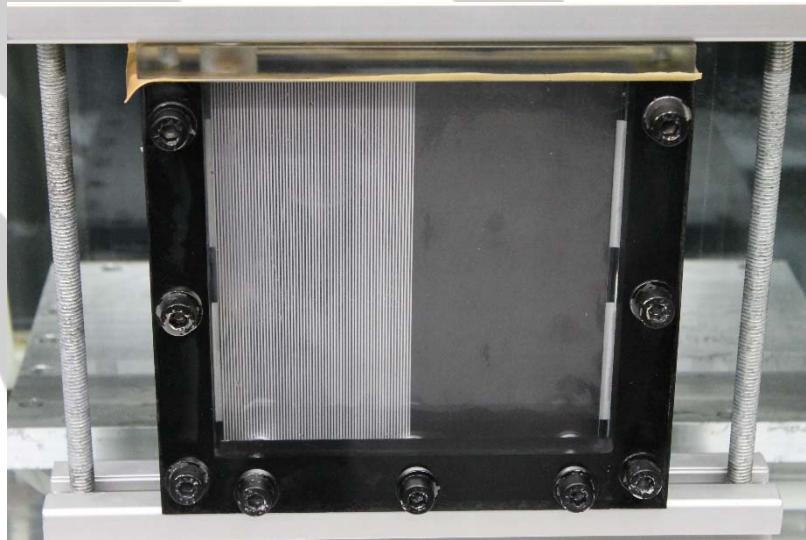


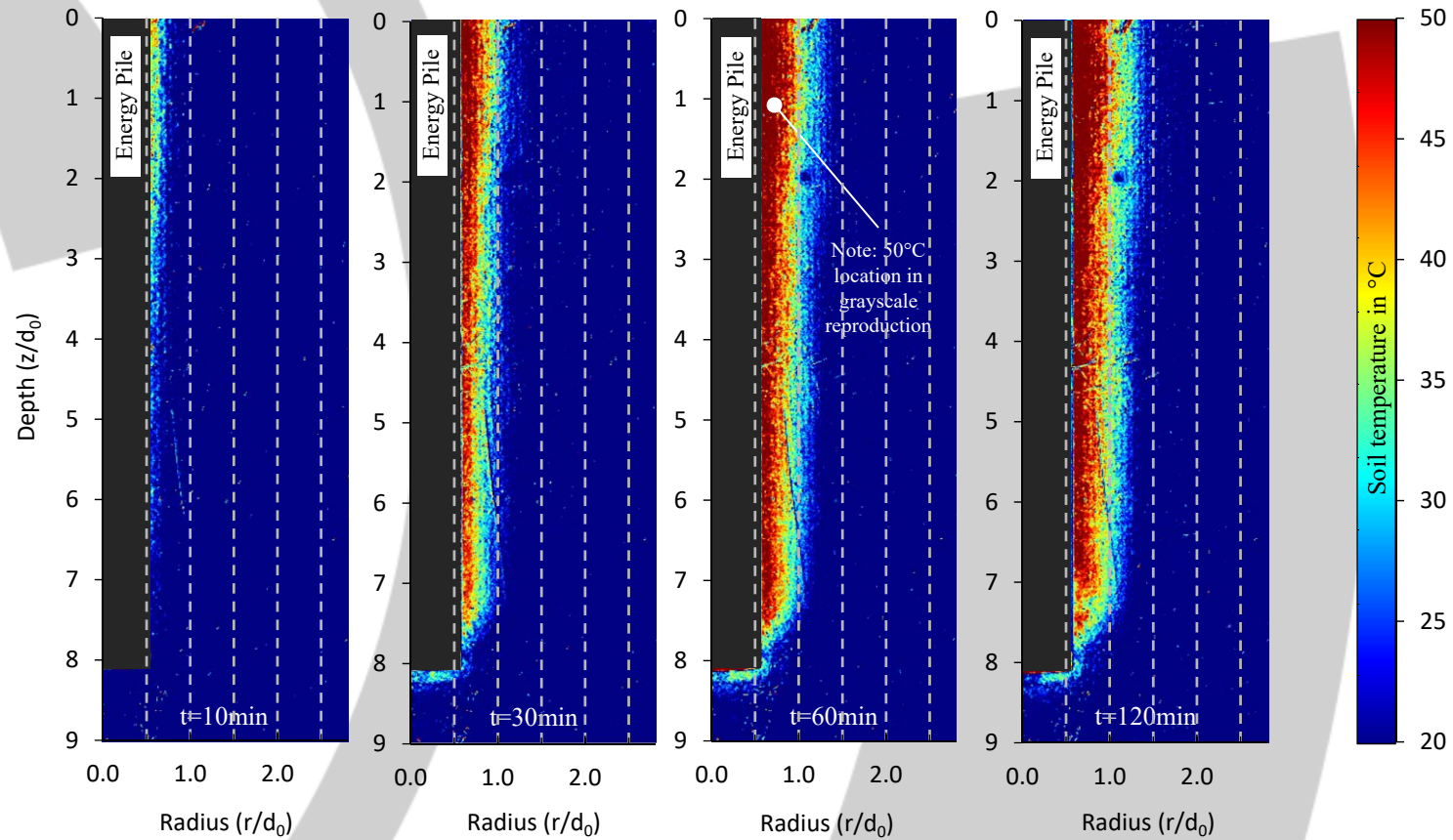


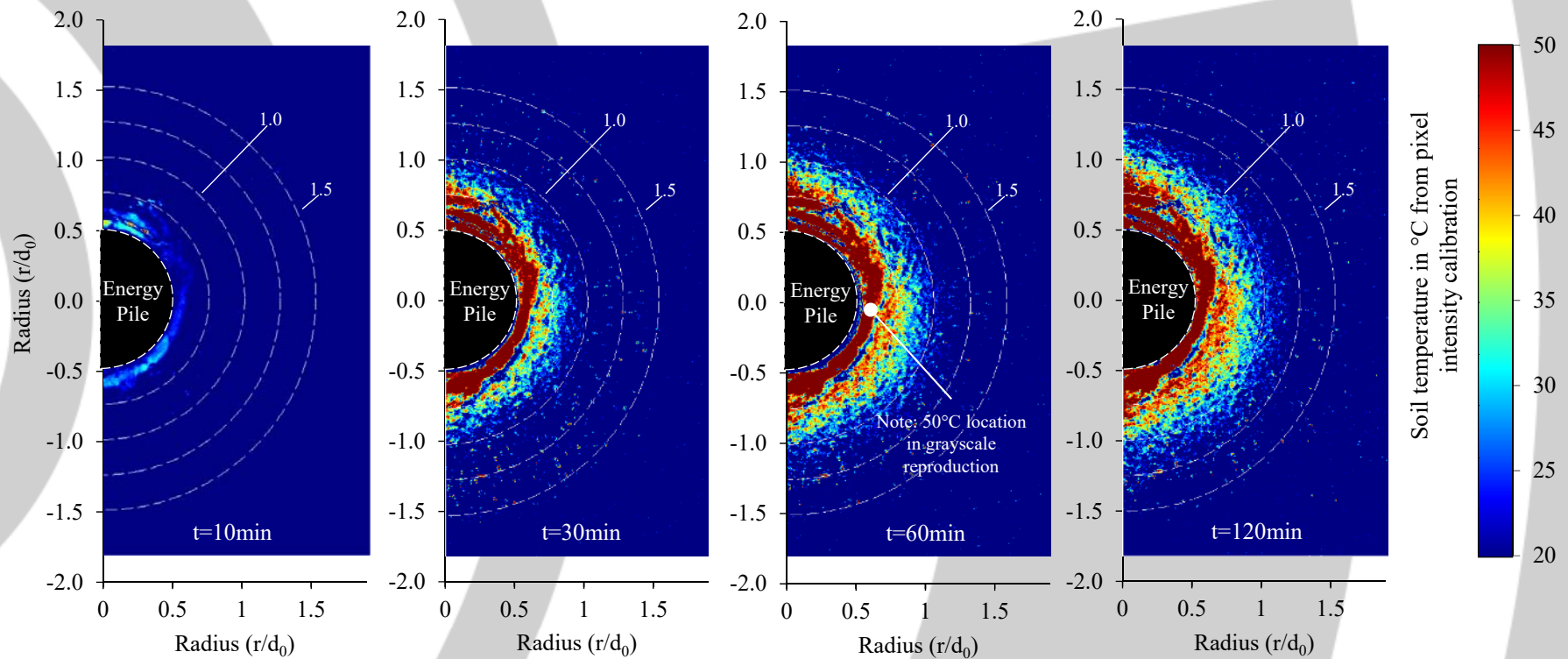
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Modelling with Transparent Soil

Thermal Modelling









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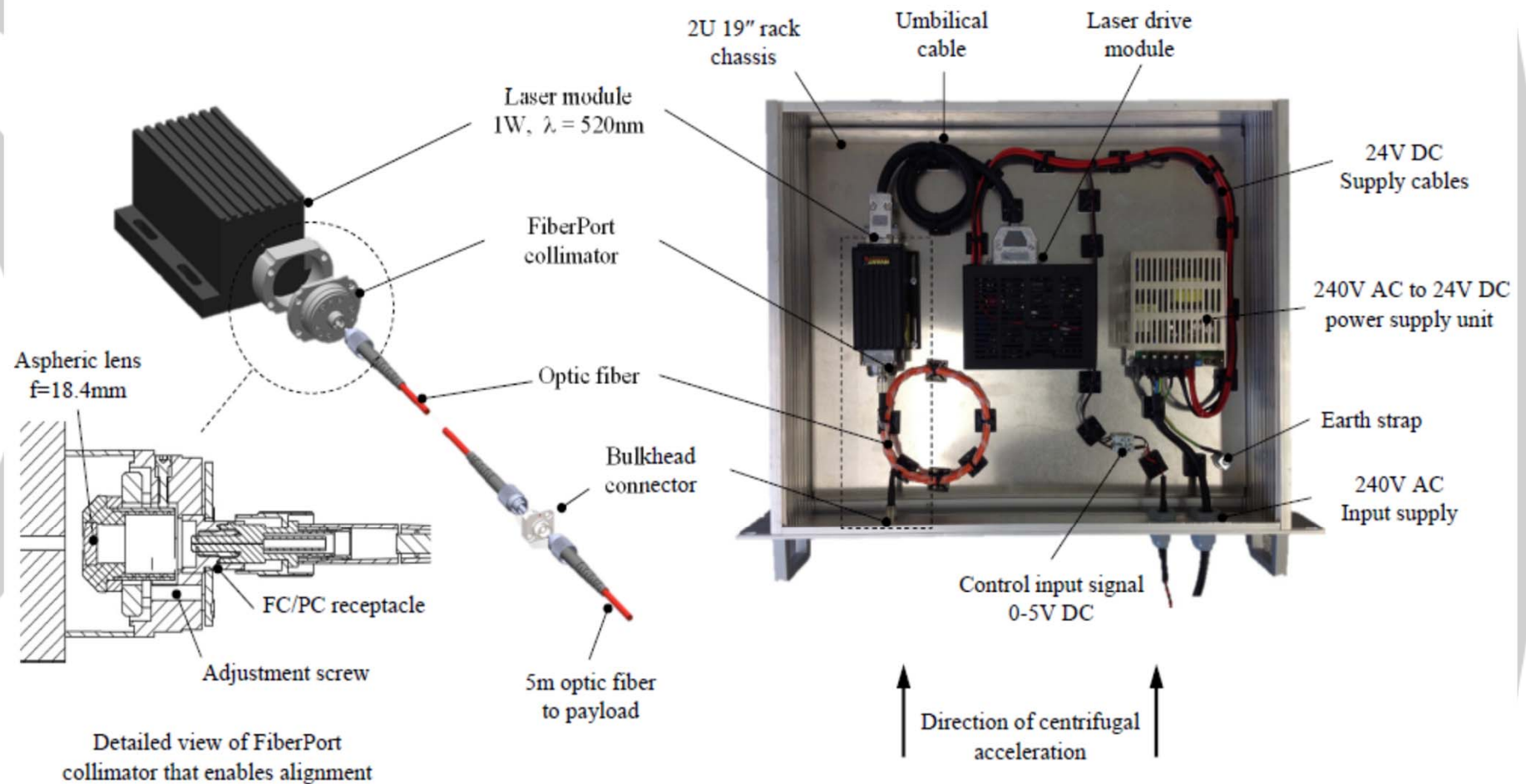
Centrifuge tests





Modelling with Transparent Soil

Centrifuge tests

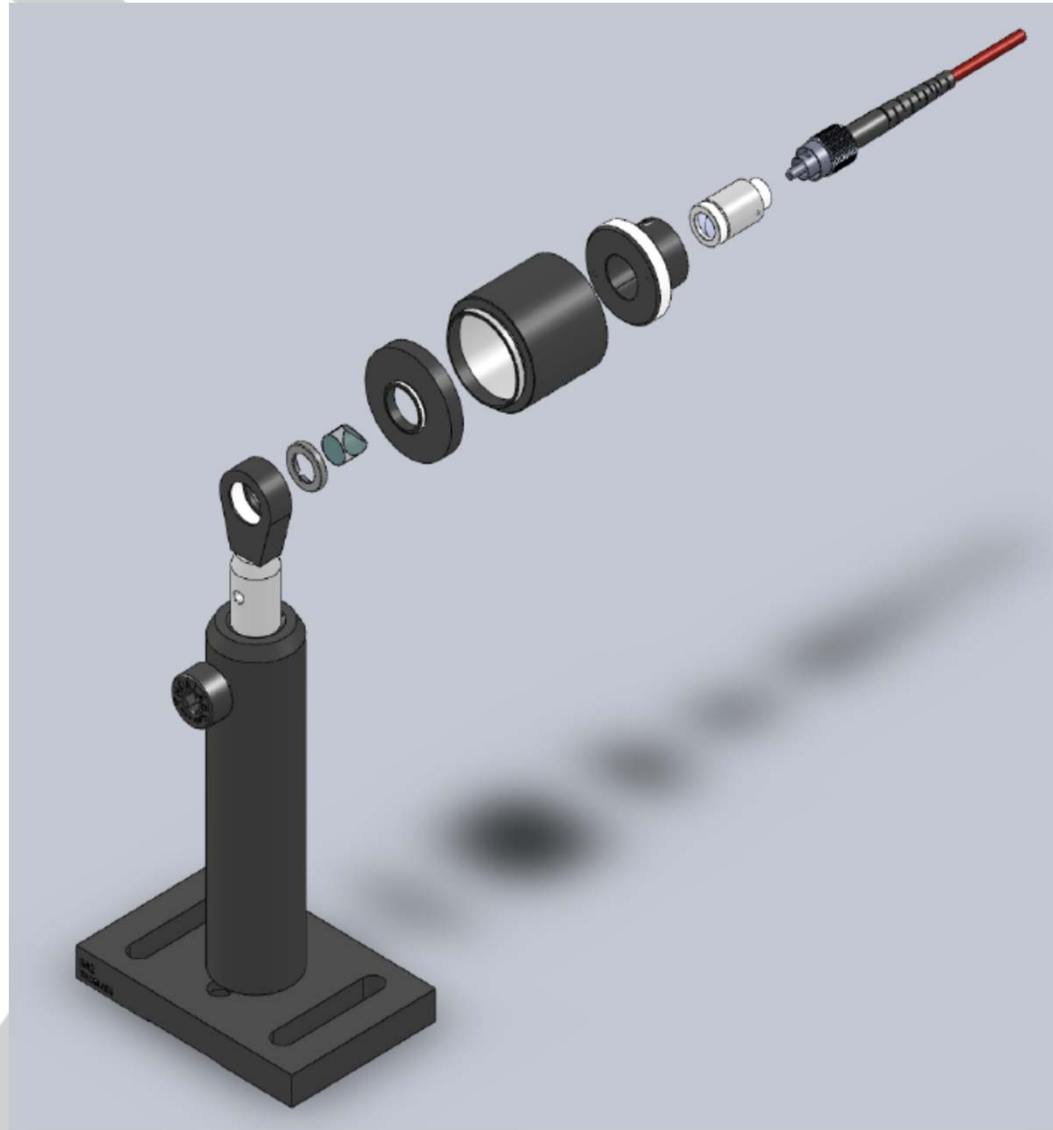




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Modelling with Transparent Soil

Centrifuge tests

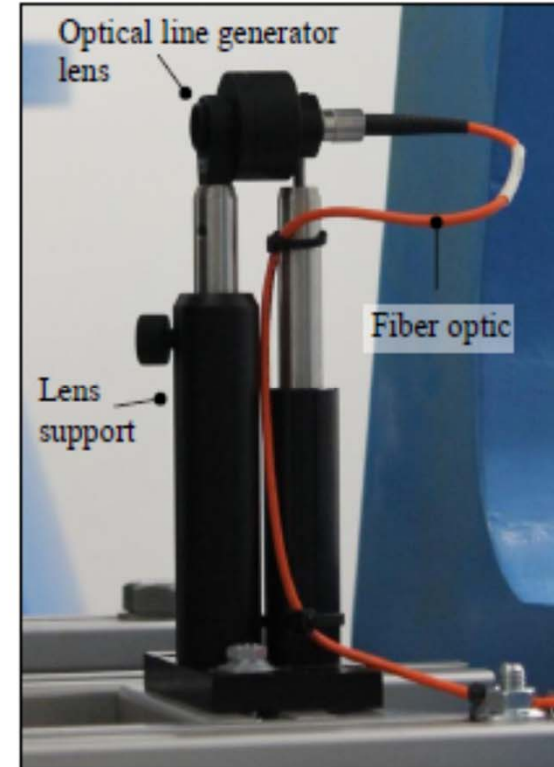
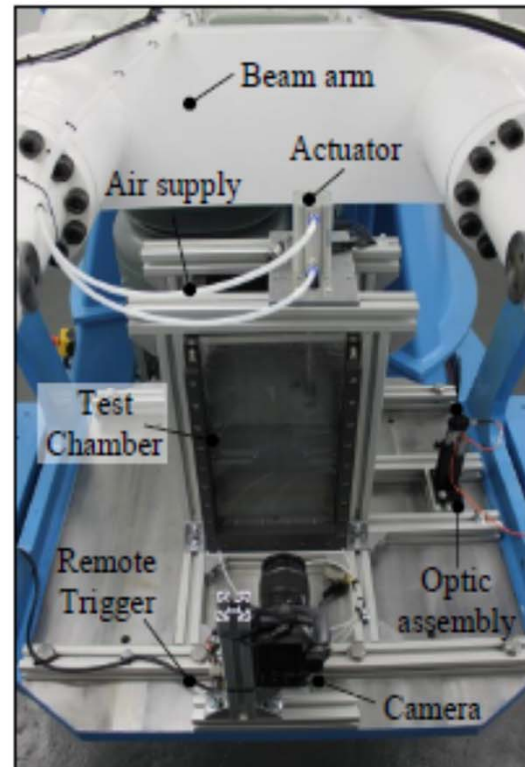


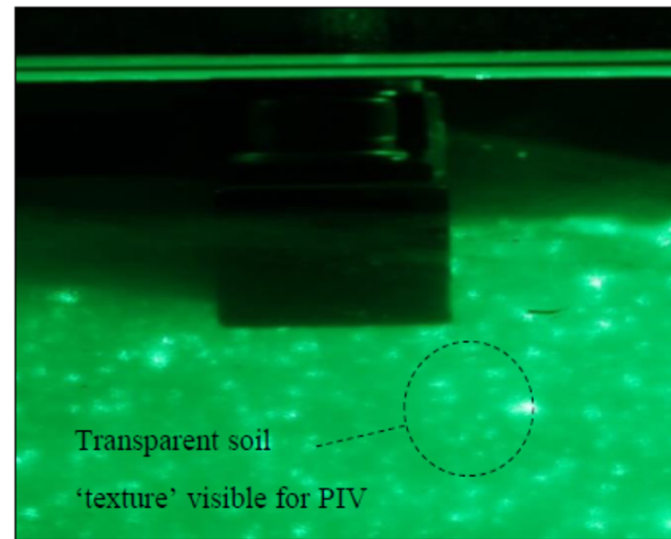
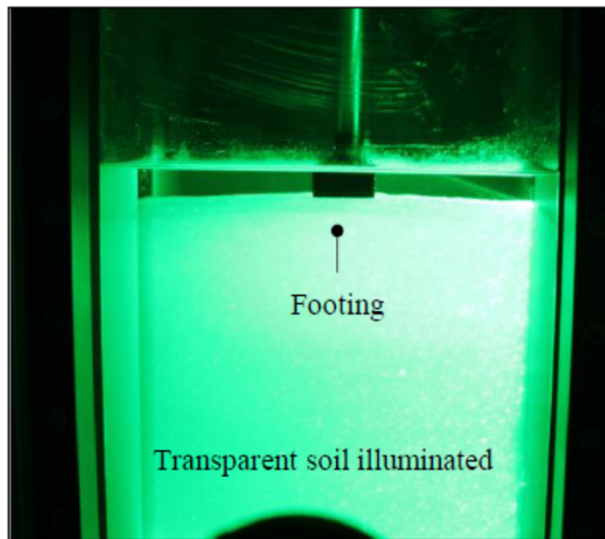
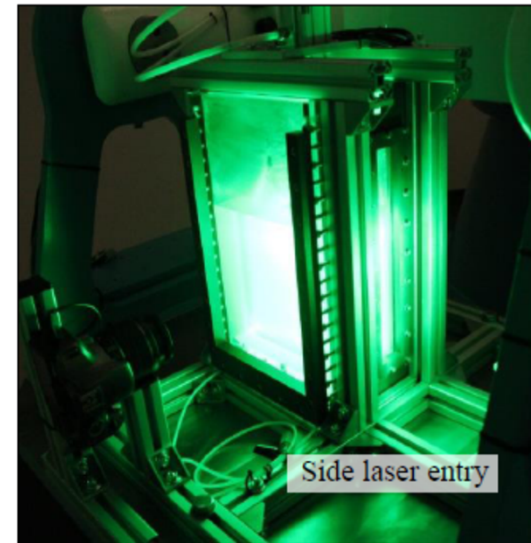
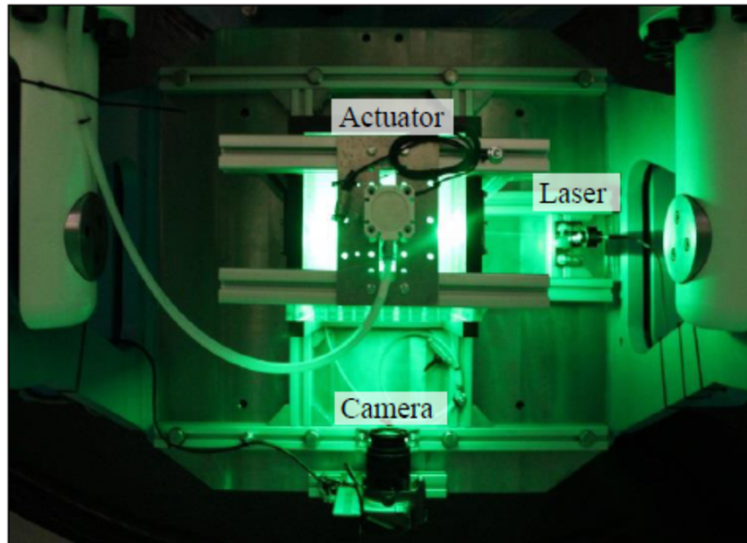


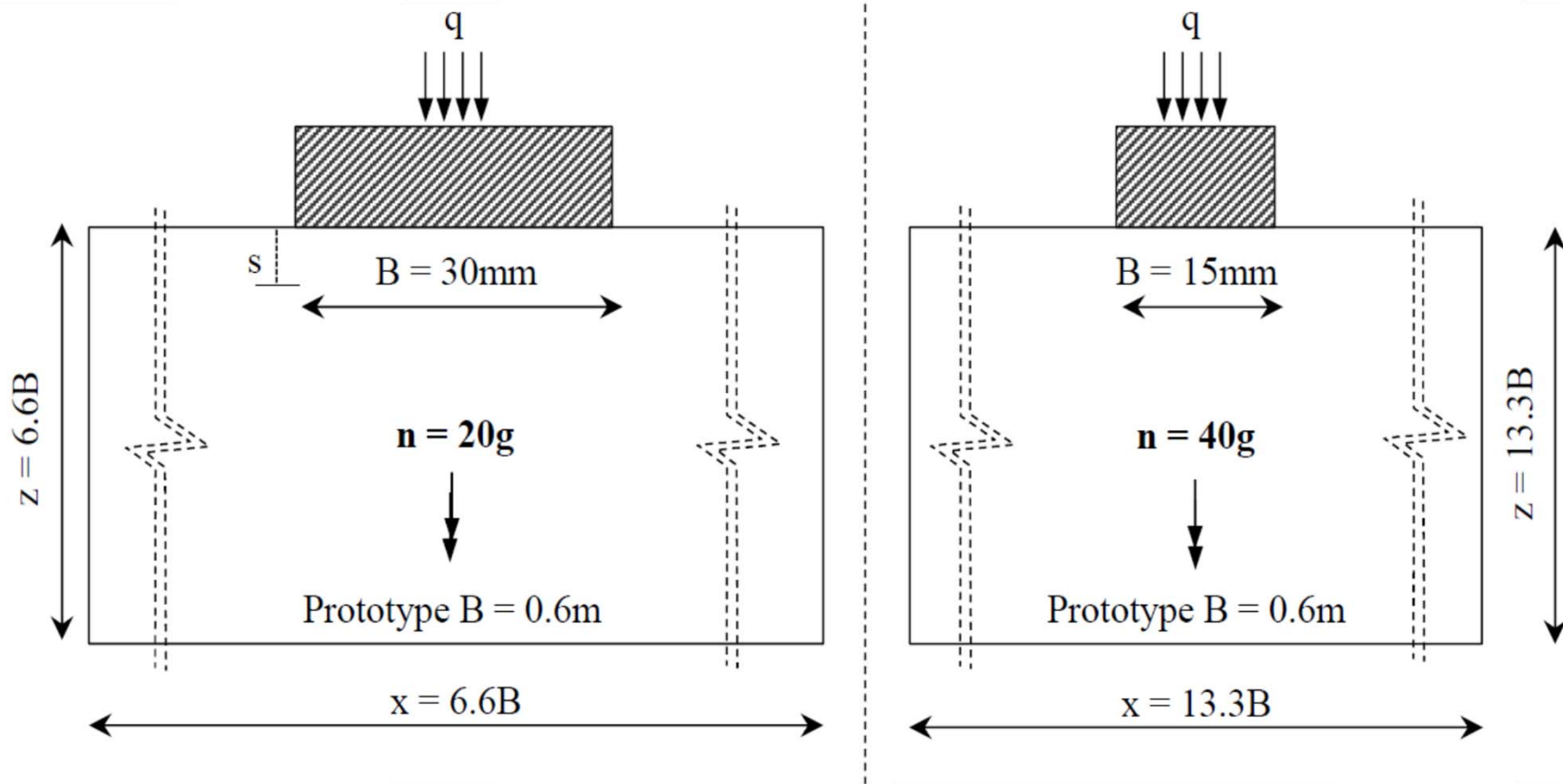
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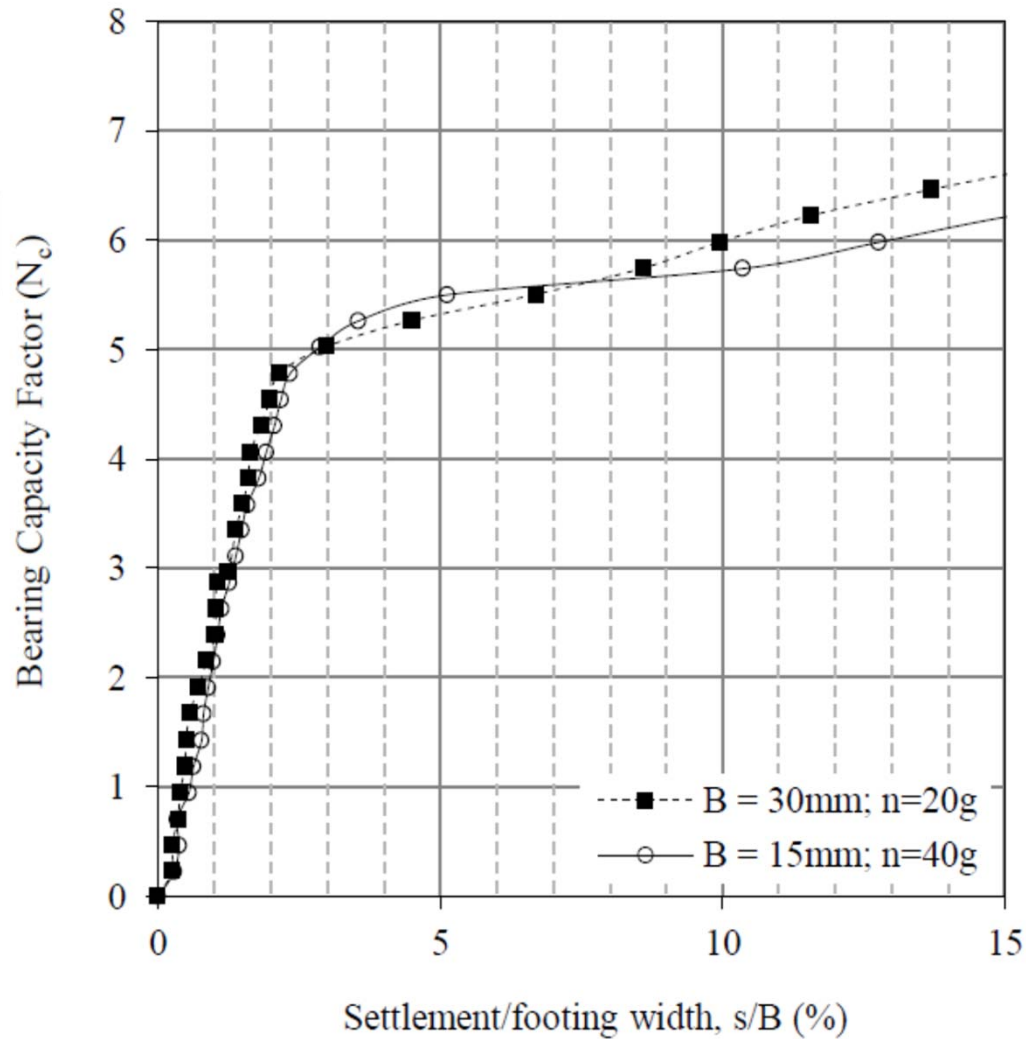
Modelling with Transparent Soil

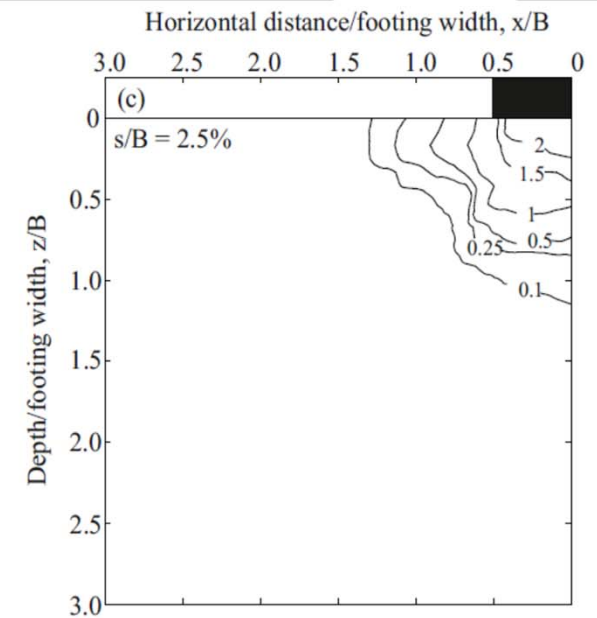
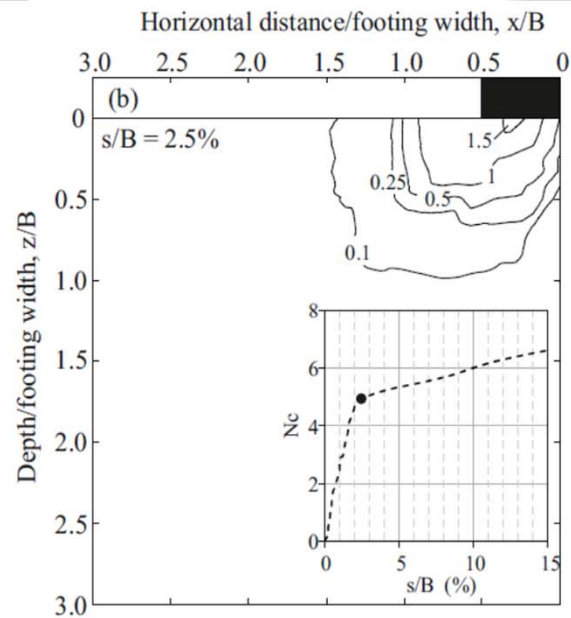
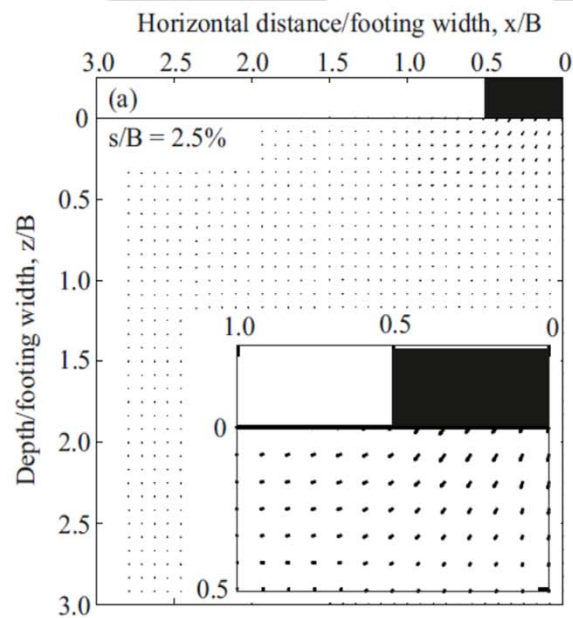
Centrifuge tests

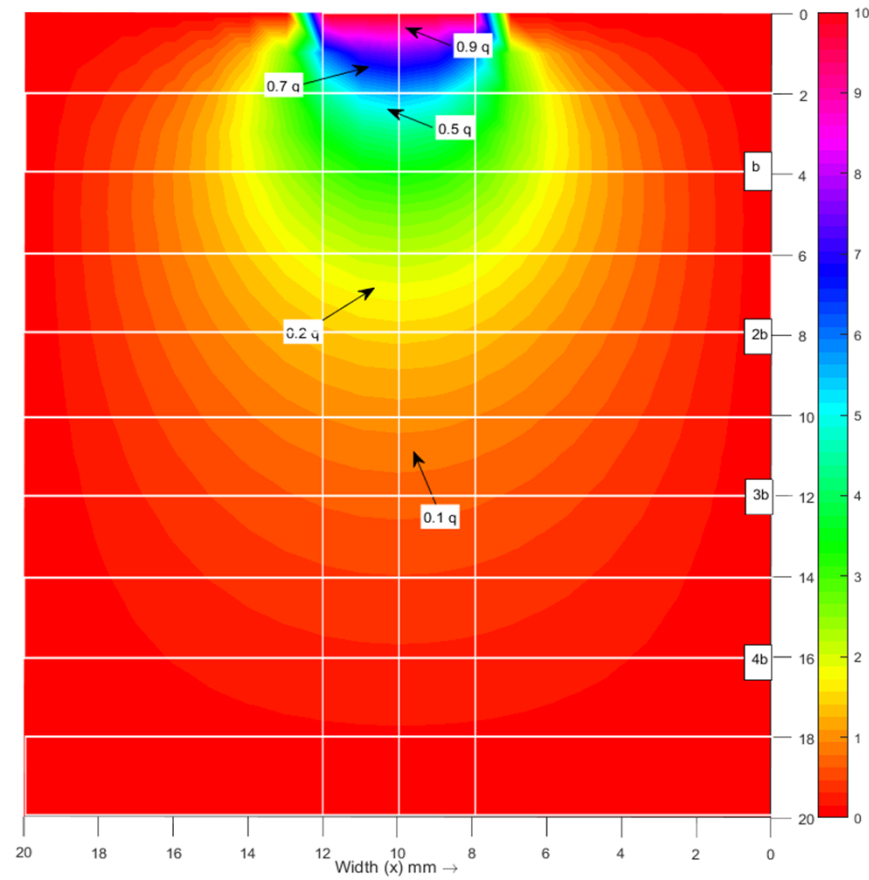
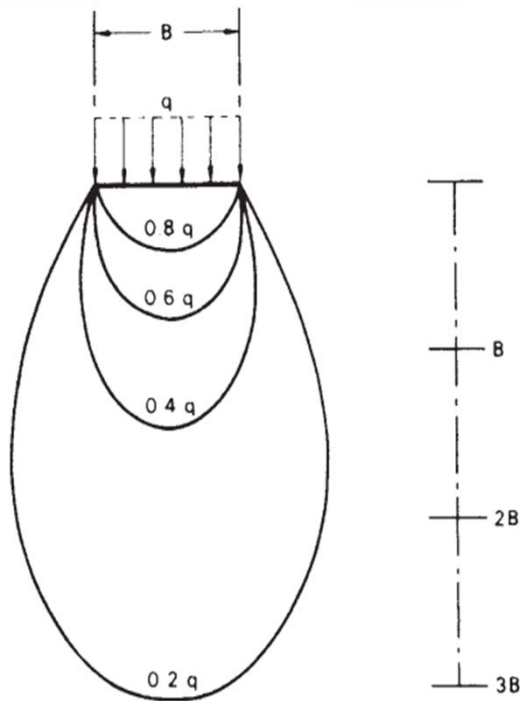


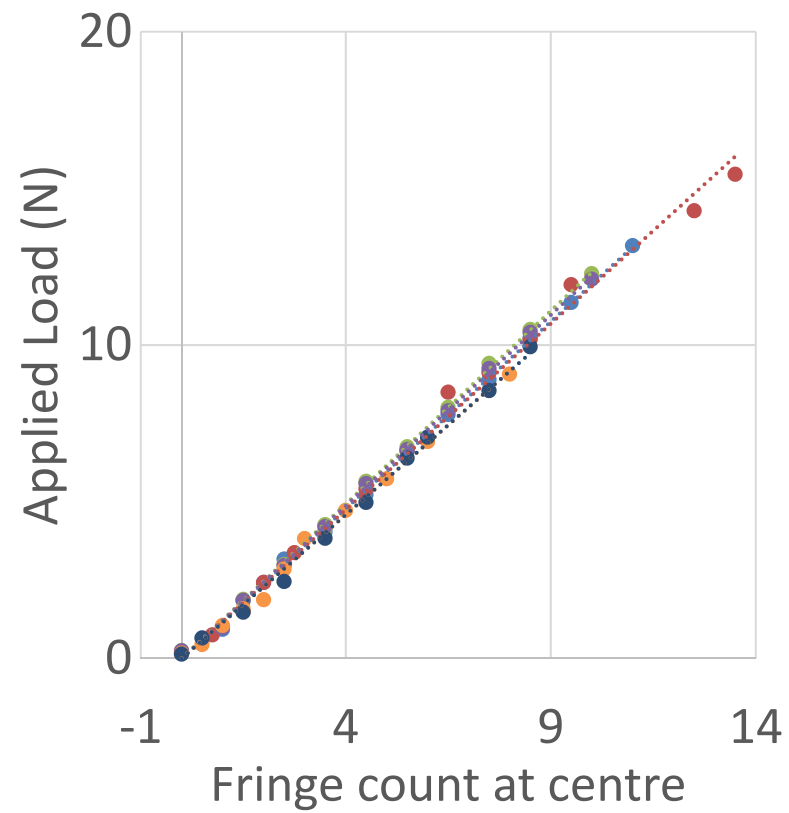
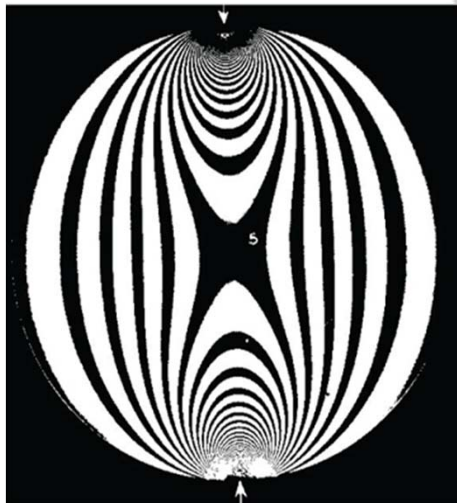
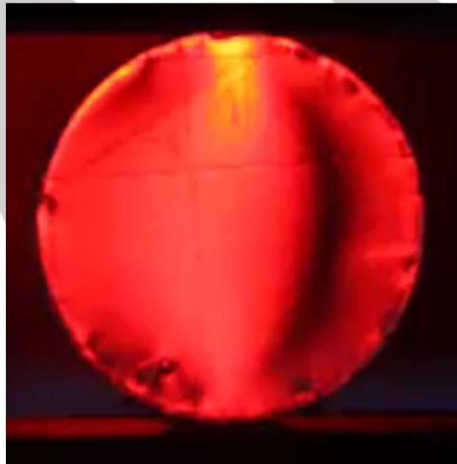










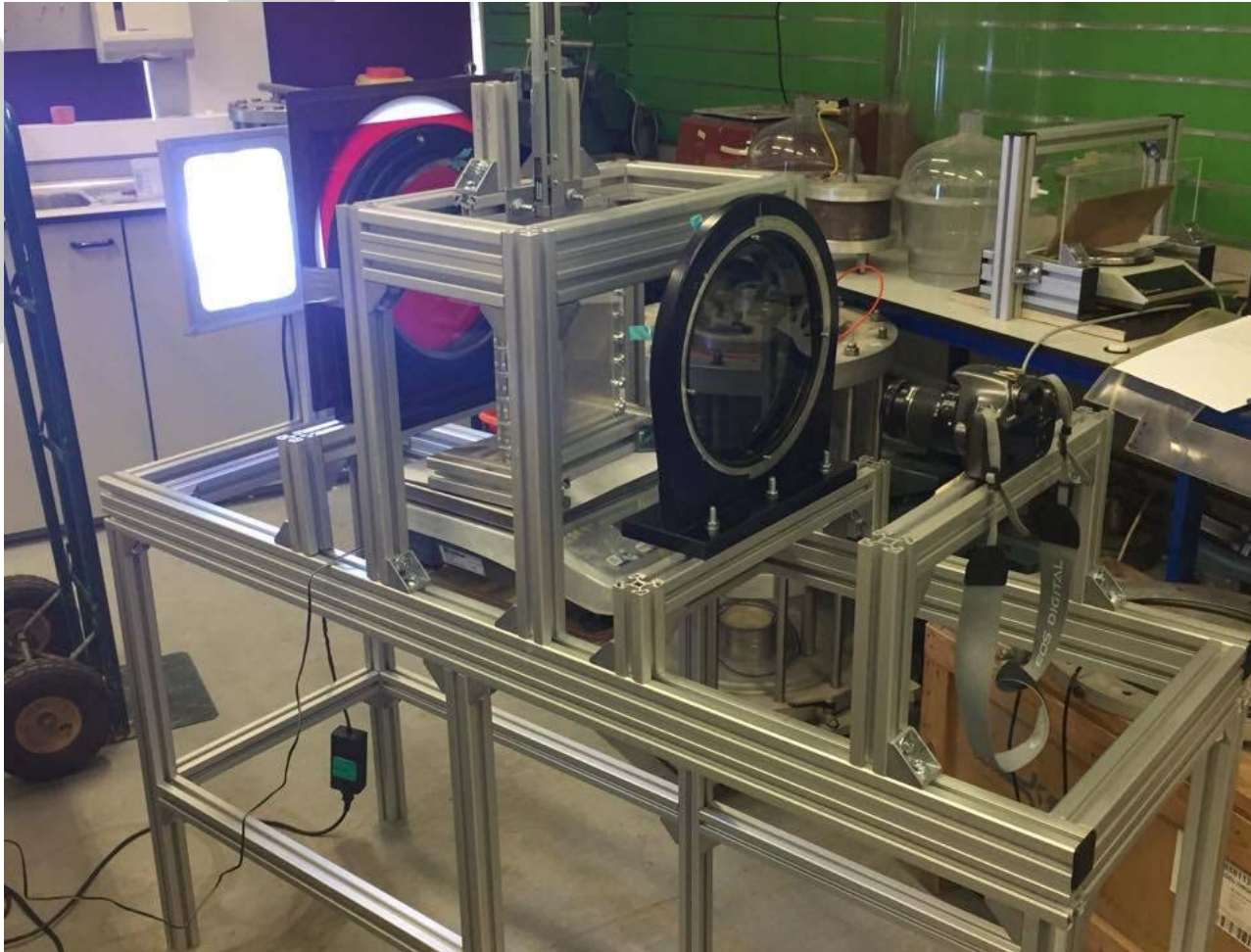




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Modelling with Transparent Soil

Stress-strain

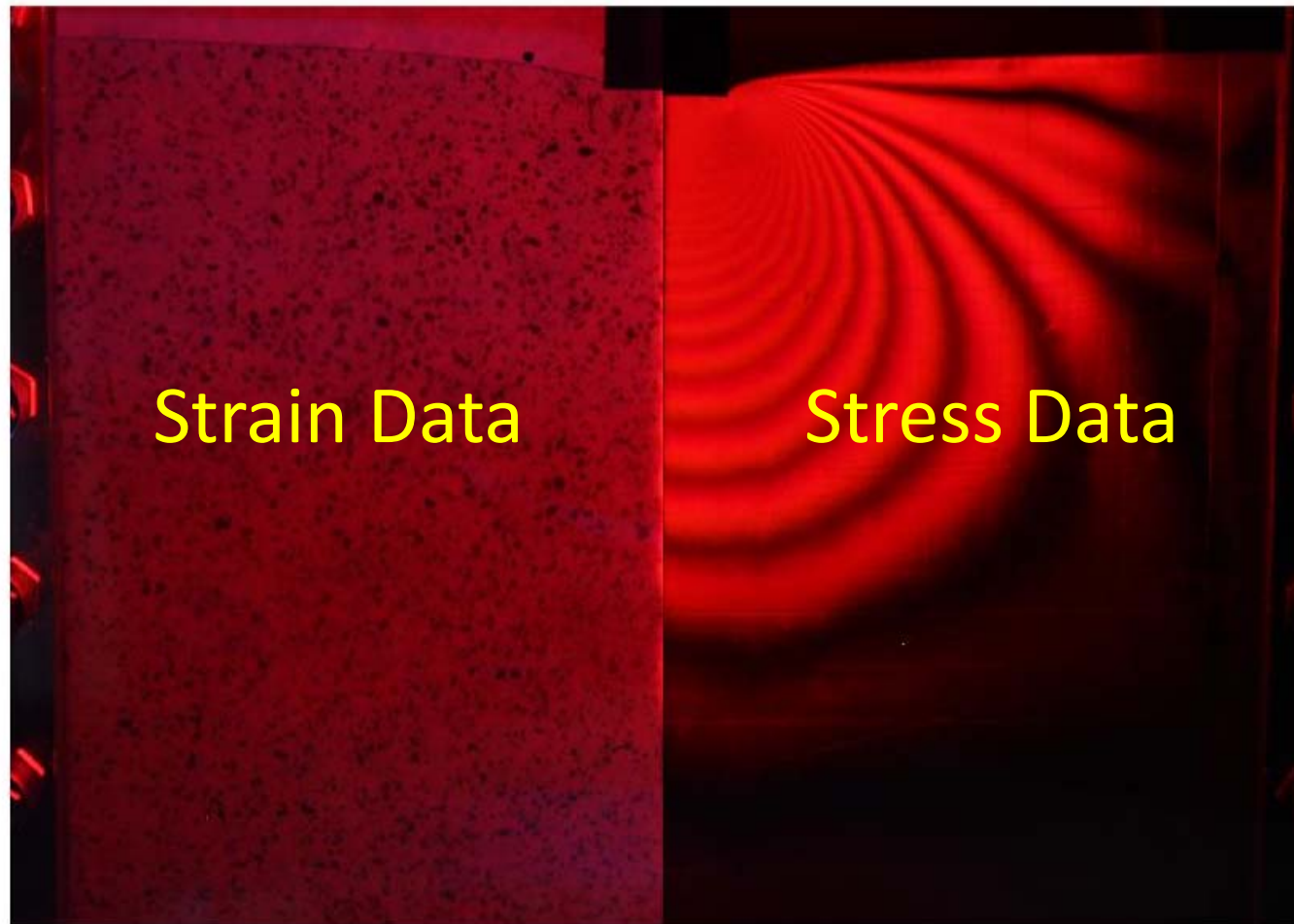




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Modelling with Transparent Soil

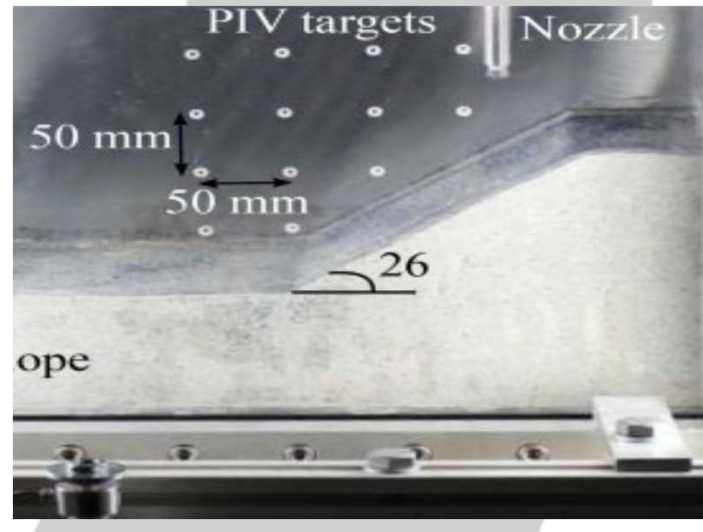
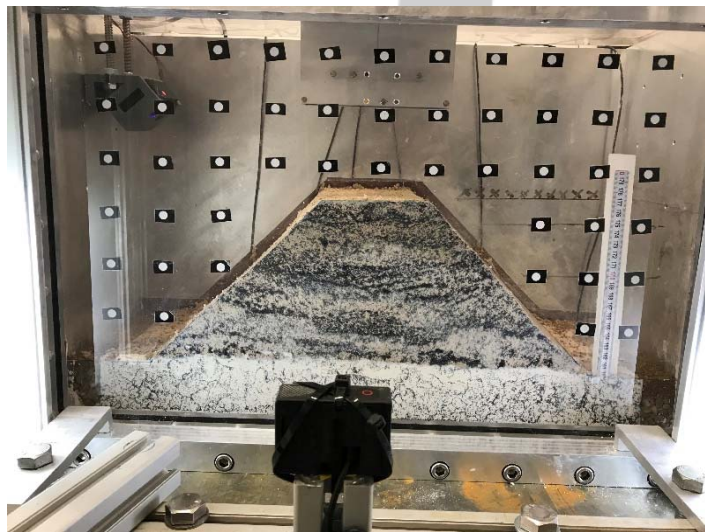
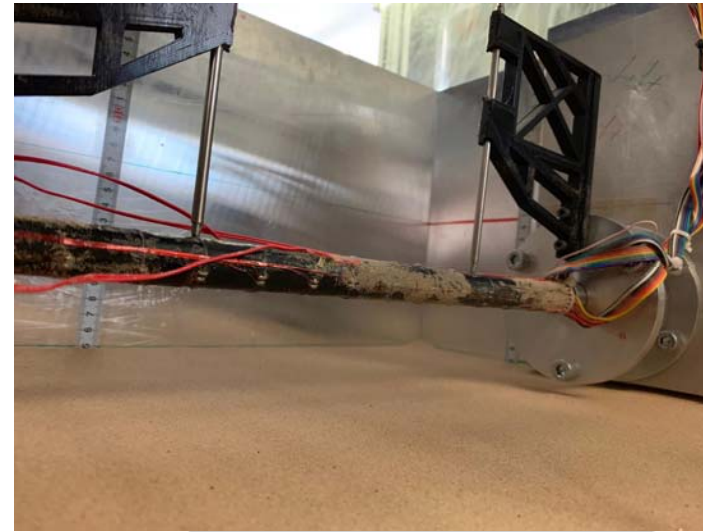
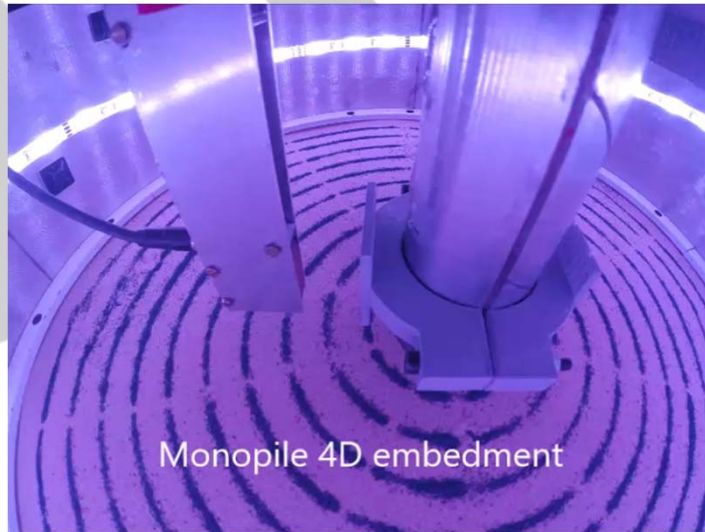
Stress-strain



Strain Data

Stress Data







- Educational impacts from centrifuge modelling to UG/PG students can be significant to learning experience
- Technology continually evolving – new opportunities
- Keep it simple!
- Advanced visual methods are unlocking new insight and understanding
- Limited only by our imagination and funding budgets





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