

1 **Laboratory experiments on the determination of porosity of granular soils**
2 **from Ontario, Canada, using electrical resistivity measurements**

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6
7 **ABSTRACT**

8 The difficulty of assessing soil properties such as relative density due sample disturbance
9 and the expensive methods of obtaining undisturbed field samples of granular soils has increased
10 the need for finding inexpensive in-situ testing methods. The approach of using geophysical
11 techniques by measuring the electrical resistance of cohesionless soil can be used to define many
12 geotechnical parameters and physical properties of sand without the need to obtain field samples
13 for laboratory tests, thereby minimizing the effects of soil disturbance. This paper presents an
14 experimental testing program for investigating the effects of pore water salinity, soil fabric,
15 porosity, and fines content on electrical resistivity of saturated sands. The results show that the
16 electrical resistivity of saturated sands decreases with increasing porosity, or increasing
17 electrolyte concentration and fines content at a certain porosity, while sand fabric and gradation
18 have relatively minor influence. Empirical correlations are thus developed for estimating sand
19 porosity and hydraulic conductivity. The proposed correlations would be useful engineering
20 tools to determine the in-situ porosity and seepage characteristics of sands.

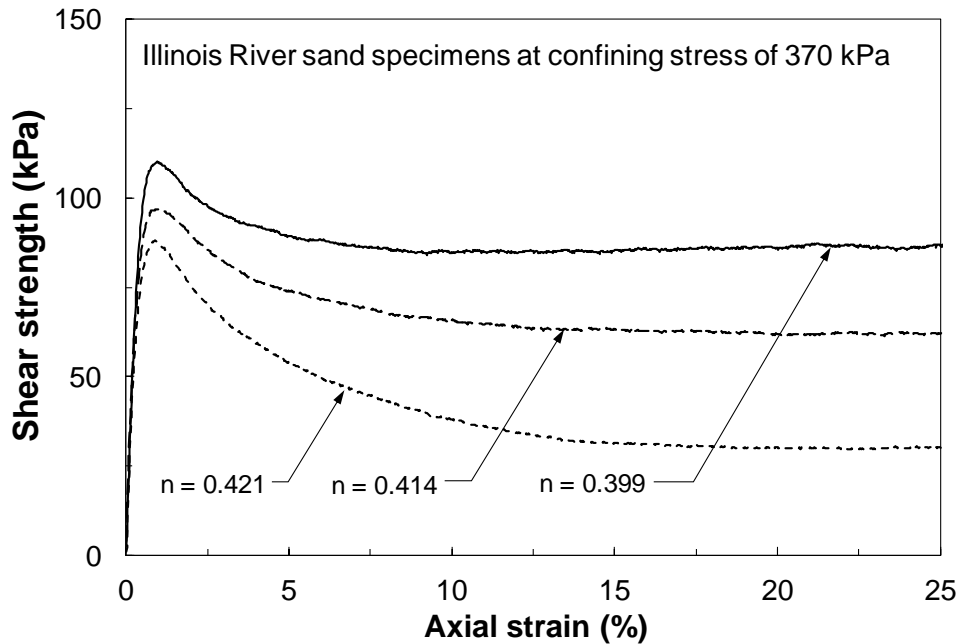
21 **Keywords:** Electrical resistivity, fines content, porosity, permeability, formation factor, soil,
22 Ontario, Canada

23 **Introduction**

24 Shearing strength and the mechanical behavior of saturated cohesionless soils is very important
25 in the stability of slopes, tailings dams, bearing capacity of foundations, and the stability and
26 design of soil retaining structures. Slope failures and landslides often results from the inadequate
27 strength of soil compared to the applied shear stress. For example, the Merriespruit gold mine
28 tailings dam in Virginia, South Africa was constructed by the hydraulic filling of loose mine

29 tailings. As a result of increasing the applied shear stress by oversteepening, the tailings dam
30 experienced slope failure followed by a massive flowslide failure in February 1994 which
31 released 600,000 m³ of waste tailings over a distance of more than 2,000 m, killed 17 people, and
32 destroyed 280 houses (Fourie, et al., 2001). More recently, a catastrophic landslide occurred on
33 the 22th of March 2014 in Oso, Washington (USA) after three weeks of intense rainfall. The Oso
34 landslide mass obliterated more than 50 homes, claimed 43 lives, injured 10 people, and buried
35 portions of a major state highway leading to an estimated capital loss of at least \$50 million. The
36 failure occurred in a loose sandy colluvial material susceptible to undrained shearing failure and
37 static liquefaction (Keaton, et al., 2014). Despite considerable advances in understanding
38 landslide mechanisms, these phenomena continue to cause significant damage throughout the
39 world partly due to the extreme difficulties and large expenses associated with undisturbed
40 sampling of saturated cohesionless soils for assessing their in-situ strength and determining their
41 susceptibility to failure.

42 The mechanical behavior and shear strength of cohesionless soils subject to shear stress (e.g. in a
43 laboratory triaxial shear test, beneath a slope, behind a retaining wall, or within an earth or
44 tailings dam) are largely controlled by their density and porosity (Sadrekarimi, 2013). For
45 example, Figure 1 illustrates the strong effect of porosity on the shearing behavior and the
46 strength of Illinois river sand specimens in triaxial compression shear tests (Sadrekarimi, 2009).
47 The undrained strength obtained from triaxial compression tests on soil specimens reconstituted
48 at the in-situ porosity could then be extended to the field for assessing the stability of slopes, and
49 the design of embankment or tailings dams. Therefore, determination of in-situ porosity and thus
50 density of sands is essential for predicting the in-situ shearing strength and liquefaction
51 susceptibility behavior, densification control, as well as determining seepage characteristics of
52 cohesionless soils. These parameters at the top 10 m of a seabed are particularly important for
53 the design of most offshore structures (e.g. oil platforms, wind turbine foundations, oil
54 pipelines).



55

56 Figure 1: Effect of porosity (n) on the undrained shearing behavior of Illinois river sand in
 57 triaxial compression tests (Sadrekarimi, 2009)

58

59 However, direct measurement of the in-situ porosity and density of cohesionless soils is
 60 challenging due to the difficulties in obtaining undisturbed samples for laboratory testing and the
 61 susceptibility of cohesionless soil samples to disturbance caused by borehole excavation,
 62 sampling, during transportation, and sample extrusion and handling. In particular, saturated
 63 sands are notoriously difficult to sample without disturbance. Even carefully collected, thin-wall
 64 cores are likely to collapse during sampling or sample extrusion resulting in an underestimation
 65 of the in-situ porosity. This becomes further complicated by the inherent variability of field soil
 66 deposits which makes even the few high-quality undisturbed samples inadequate for
 67 characterizing subsurface soil conditions. These challenges have increased the need of finding
 68 more reliable and cost-effective in-situ testing methods. Geophysical techniques and in particular
 69 soil electrical resistivity measurements, have been used for examining the in-situ porosity
 70 (Barnes, et al., 1972, Erchul and Nacci, 1971, Jackson, et al., 1978, Kermabon, et al., 1969,
 71 Taylor-Smith, 1971, Wheatcroft, 2002) and permeability (Abu-Hassanein, et al., 1996, Jones and
 72 Buford, 1951, Kosinski and Kelly, 1981, Urish, 1981) of sediments, characterizing the degree of
 73 soil compaction (Abu-Hassanein, et al., 1996) and consolidation behavior (Bryson and Bathe,

74 2009, Cho, et al., 2004, Kim, et al., 2011, Lee, et al., 2008) without obtaining field samples, and
75 thus minimize soil disturbance effects. For example, Kermabon et al. (1969) developed a field
76 electrical resistivity probe for predicting the in-situ porosity of marine sediments, composed of
77 interbedded clay and sand layers. Wheatcroft (2002) used an in-situ resistivity probe to measure
78 the near-surface porosity of shallow-water marine sediments off Florida and Bahamas. The
79 measurements indicated a thin zone of higher porosity near the surface of seabed, small-scale
80 porosity fluctuations in the subsurface, and horizontal variation in the near-surface porosity. Cho
81 et al. (2004) used electrical resistivity for measuring soil water content and the consolidation
82 process of a clay soil. Lee et al. (2008) used electrical resistivity measurements for monitoring
83 the consolidation behavior of clays and estimating the preconsolidation pressure. Bryson and
84 Bathe (2009) developed a multi-electrode cell to measure the bulk electrical resistivity and
85 anisotropy of compacted sand-clay soil mixtures at different volumetric water. Kim et al. (2011)
86 developed a four-electrode resistivity probe for the measurement of porosity variation during
87 consolidation of a mixture of kaolinite clay and crushed sand. The objective of this study is to
88 examine relationships between electrical resistivity, permeability, and porosity of cohesionless
89 soils and the factors affecting these relationships.

90 .

91 **Theoretical Basis**

92 Electrical resistivity, ρ (ohm·m) of a material is a measure of how well the material
93 allows the flow of an electrical current through it. As particles (composed of quartz, feldspar, or
94 carbonates) of a cohesionless soil are often non-conductive and act as insulators, the flow of an
95 electrical current through a saturated granular soil occurs primarily through the pore water.
96 Therefore, conductivity, volume, and the distribution of the pore water control the bulk electrical
97 resistivity of sands (ρ_b). Accordingly, ρ_b can be directly related to the pore water resistivity (ρ_f)
98 and pore volume (characterized by soil porosity n) using the following general empirical
99 relationship (Archie, 1942, Winsauer, et al., 1952):

100

$$101 \frac{\rho_b}{\rho_f} = an^{-m} \qquad \text{Equation [1]}$$

102

103 where exponent m and coefficient a are fitting parameters which depend on pore volume
104 geometry and soil particle cementation (Schon, 2004). These parameters can be determined from
105 laboratory calibration experiments. The exponent, m varies from 1.0 – 1.5 for clean sands to 1.9
106 – 3.3 for clayey soils (Atkins and Smith, 1961, Barnes, et al., 1972, Campanella and Weemees,
107 1990, Carothers and Porter, 1971, Erchul and Nacci, 1971, Jackson, et al., 1978, Salem and
108 Chilingarian, 1999, Taylor-Smith, 1971). The coefficient a is found to vary within 1.2 – 1.4 for
109 loosely deposited young sands and tends to increase with increasing the density and age of a
110 deposit (Boyce, 1968, Carothers and Porter, 1971, Erchul and Nacci, 1971, Schon, 2004). For
111 example, Erchul and Nacci (1971) found $a = 1.3$ and 1.5 for loose and dense Ottawa sand
112 samples, respectively. The ratio ρ_b/ρ_f is known as the “formation factor” (FF) which expresses
113 the effects of changes in pore water resistivity as a result of the presence of non-conductive sand
114 particles (Schon, 2004). Similar to ρ_b , FF is an intrinsic property of a soil which depends on the
115 volume and the geometry of soil pore spaces.

116 Several investigators have found reasonable agreement between electrical resistivity and porosity
117 measurements with Equation [1] (Erchul and Nacci, 1971, Hulbert, et al., 1982, Jackson, et al.,
118 1978). For example, Erchul and Nacci (1971) investigated the changes in the electrical resistivity
119 of different soil types (including an illite clay, a kaolinite clay, Providence silt, Ottawa sand with
120 rounded particles, a glacial sand with angular particles, and a marine sediments) with varying
121 interstitial pore water salinity. Their findings suggest that porosity can be predicted within $\pm 2\%$
122 on the basis of the formation factor measurements made in the laboratory using Equation [1].
123 Jackson et al. (1978) investigated unconsolidated marine sands using laboratory electrical
124 resistivity measurements. They found that the formation factor – porosity relationship for
125 unconsolidated marine sands was governed by Equation [1]. They also observed that the
126 exponent m was sensitive to the shape of the particles, and it increased as the particles became
127 angular. Erickson and Jarrard (1998) examined the relationship between porosity and electrical
128 resistivity of shallow silica sediments from the Amazon Fan and found that muds and sands
129 exhibit different trends of porosity and formation factor due to differences in pore volume and
130 tortuosity. Accordingly, Equation [1] has been historically the first practical relationship between
131 a readily measurable soil property and soil porosity (Schon, 2004).

132
133

134 **Experimental Program**

135 The experiments of this study were aimed to determine the effects of fines content, sand fabric,
136 salt concentration and porosity on the electrical resistivity of sands as discussed in the following
137 paragraphs.

138

139 *Sample preparation*

140 As illustrated in Figure 2, sand samples were prepared using undercompaction moist tamping in
141 a cylindrical acrylic chamber with an internal diameter and height of 114.4 mm and 250 mm,
142 respectively. The samples were then saturated by soaking them with water for 24 hours in order
143 for the water to percolate the pore spaces among sand particles. An average degree of saturation
144 of 96% was confirmed based on the volume and weight measurements of saturated samples. In
145 the moist tamping method, the dry sand was premixed with 5% moisture and thoroughly mixed
146 to uniformly distribute the moisture content. Then predetermined weights of the moist sand were
147 deposited and tamped in 5 layers of 50 mm thick using an adjustable-height handheld tamper.
148 The structure of specimens prepared by moist tamping resembles that of hydraulically
149 transported sand fills. The weight of wet soil placed in the lower layers was intentionally less
150 than those of the upper layers in order to account for the increase in soil density as a result of
151 tamping the successive overlying layers and improve specimen uniformity. The difference in
152 density between successive layers is defined as the undercompaction ratio (Ladd, 1978). Based
153 on an undercompaction ratio of 10%, the density of each layer was increased linearly from the
154 bottom to the top of the specimen. This method led to more uniform samples and repeatable test
155 results.

156

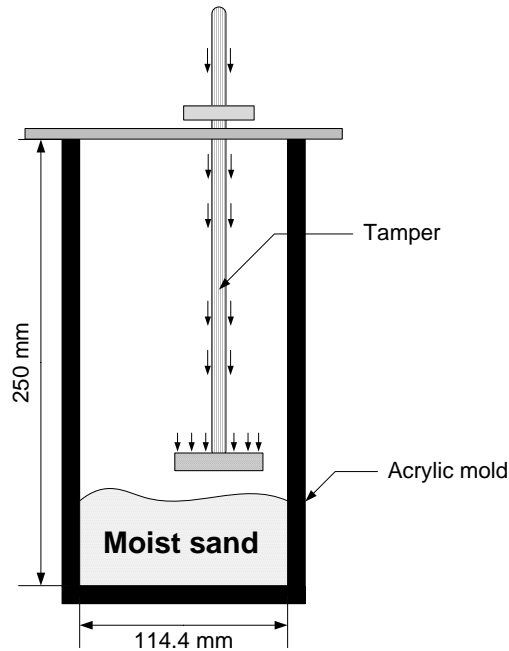


Figure 2: Schematic of the moist tamping specimen preparation method

Materials

Two gradations of Ottawa sand (called “Barco 32” and “Barco 71”) as well as a local silty sand (with a fines content of 11%) from the Boler Mountain (London, Ontario) were used in the experiments of this investigation. The particles of Barco 32 and Barco 71 sands are purely quartz with rounded to subrounded shapes, while the Boler Mountain sand is composed of subrounded to subangular particle shapes with mixed mineralogy of quartz, feldspar and carbonates. Barco 71 sand was mixed with different amounts of quartz silt particles to produce different silty sands and investigate the effect of fines content (FC). Figure 3 shows the gradations of these materials. Table 1 presents the grain density (G_s) of the sand particles as well as their minimum (n_{min}) and maximum (n_{max}) porosities. These properties were determined according to the ASTM standard procedures (ASTM, 2006a, ASTM, 2006c, ASTM, 2006d).

Table 1. Physical properties and index characteristics of the tested sands

Sand	G_s	n_{min}	n_{max}
Barco 32	2.65	0.346	0.444
Barco 71	2.65	0.332	0.451
Boler Mountain	2.65	0.310	0.486

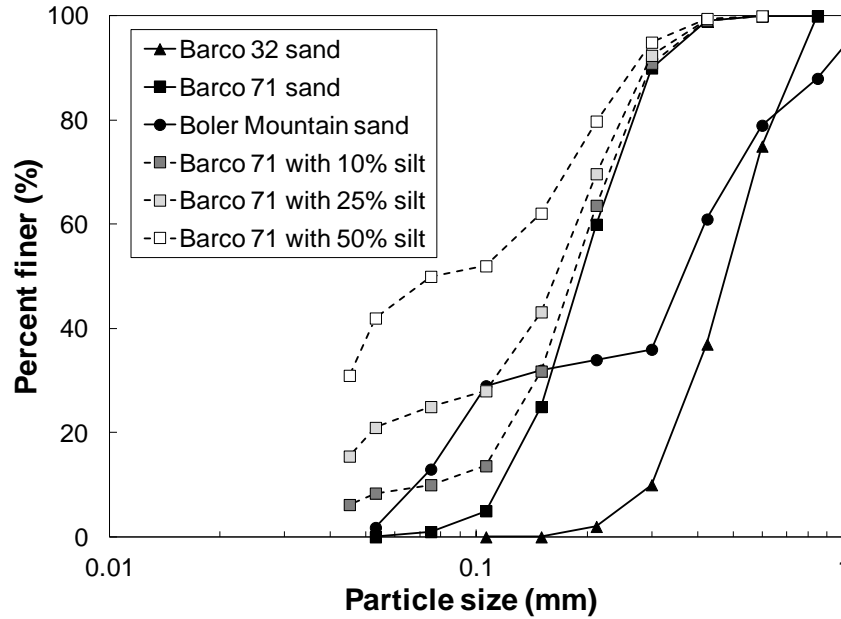


Figure 3: Particle size distributions of the sands used in this study

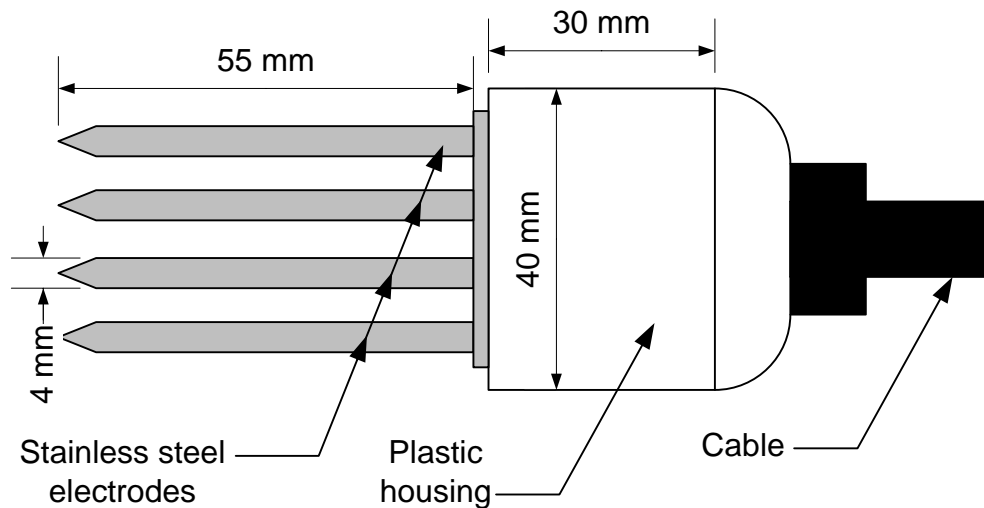
174
175
176

177 *Electrical resistivity measurement*

178 A portable electrical resistivity probe (Stevens Water Monitoring System Inc, 2007) was used to
 179 measure the electrical resistance (R) as well as the temperature of the soil water system. The
 180 probe consists of four parallel stainless steel electrodes spaced at a center-to-center distance of
 181 13 mm. Each electrode is 55 mm long and 4 mm in diameter with an apex angle of 33° . Figure 4
 182 shows the schematic shape and dimensions of the probe. The probe introduces a low-frequency
 183 (50 Hz) alternating electrical current of known intensity (I) into the soil sample at a certain depth
 184 through the electrodes, and measures the potential voltage difference (V) in the soil adjacent to
 185 the electrodes. The low alternating electrical current frequency of 50 Hz minimizes the
 186 detrimental effect of polarization of the electrodes. The average electrical resistance of the soil
 187 sample around the electrodes is then calculated from Ohm's law ($R = V/I$). These data are
 188 transferred to and displayed on a handheld PDA. Soil electrical resistivity (ρ_b), a fundamental
 189 soil property, is obtained by multiplying the measured resistance (R) by the geometrical factor, K
 190 of the probe ($\rho_b = K \times R$) which depends on the size and the separation of the electrodes. An
 191 average geometrical factor of 0.872 was determined for the probe by calibration with a TetraCon
 192 325 4-electrode conductivity cell in an ionic buffer solution of 3gr/L salt (NaCl) at a measured
 193 electrical resistivity of 2.127 ohm.m. All experiments were performed at a controlled laboratory

194 temperature of 20°C and in a non-metallic (acrylic) cylinder as metal could interfere with the
195 measurements. Four successive measurements were then taken for each sample. For each
196 measurement, the electrodes were completely inserted into the soil until the base plate of the
197 probe became flush with the soil and readings were taken until soil resistivity and temperature
198 were stabilized. These measurements were very close which indicated the high level of
199 repeatability of the measurements. The average electrical resistivity measurements of each
200 sample are reported in this paper.

201



202

203 Figure 4: Schematic shape and dimensions of the electrical resistivity probe used in this study

204

205 Results and Discussion

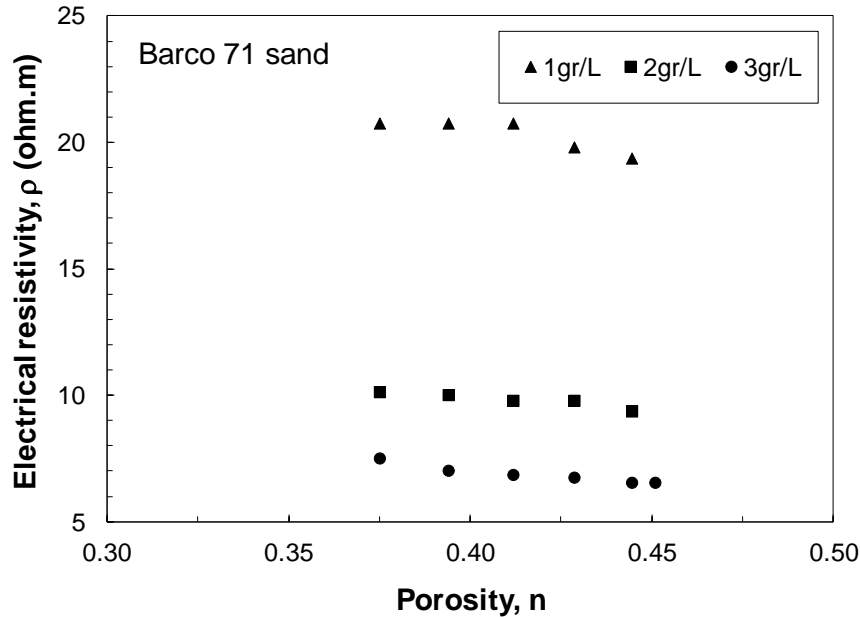
206 The results of the experiments are presented below. For each experiment, sample porosity was
207 determined based on the weight of the soil used for sample preparation and sample volume while
208 ρ_b was measured using the electrical resistivity probe.

209

210 *Effect of electrolyte concentration*

211 Electrical resistivity of the pore water (ρ_f) depends on the electrolyte concentration as a result of
212 pore water salinity, and thus at a certain porosity the electrical resistance of a saturated
213 cohesionless soil would become a function of the amount of salt in the pore fluid. Figure 5
214 presents the electrical resistivity of saturated Barco 71 sand samples with different amounts of
215 pore water salinity expressed in grams of salt per 1 liter of distilled water. Based on this figure,

216 salt content of the pore fluid electrolyte has a profound impact on the electrical resistivity of soil
217 as ρ decreases with increasing salt concentration of the pore water.
218



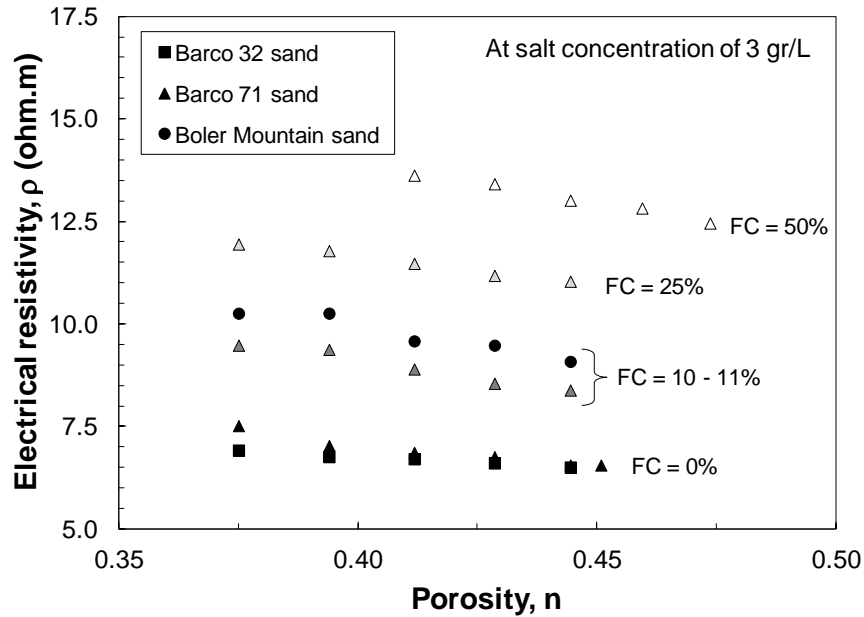
219
220 Figure 5: Effect of pore water salinity on electrical resistivity of saturated Barco 71 sand samples
221

222 Figure 5 further indicates that soil electrical resistivity is inversely proportional to its porosity.
223 As porosity increases, the electrical resistivity slightly decreases. As this investigation is focused
224 on sands with non-conductive silicate particles, increasing of the electrical conductivity is due to
225 the increase of the conducting pore water volume among the sand particles and the ability of the
226 electrolyte water to conduct electricity, reducing the electrical resistivity of the saturated sand.

227
228 *Effect of particle size distribution*

229 Figure 6 compares the electrical resistivity of the different sands tested in this study, which
230 clearly indicates increasing soil electrical resistivity with increasing of the amount of fines.
231 Increasing fines content, even at the same porosity, reduces the number and the volume of
232 connections among the pores of a sand and thus the electrical connectivity and conductivity of
233 the pore fluid. Despite differences in particle size distributions (see Fig. 3), the Barco 32 and
234 Barco 71 sands exhibit nearly similar electrical resistivity at FC = 0%, or the Boler Mountain and
235 Barco 71 sands with 10 to 11% fines contents show similar electrical resistivity. Hence, the

236 effect of variations in particle size distribution on the electrical resistivity of sands seems to be
 237 secondary to the effect of FC. A similar observation was made by Keller and Frischkecht (1966).
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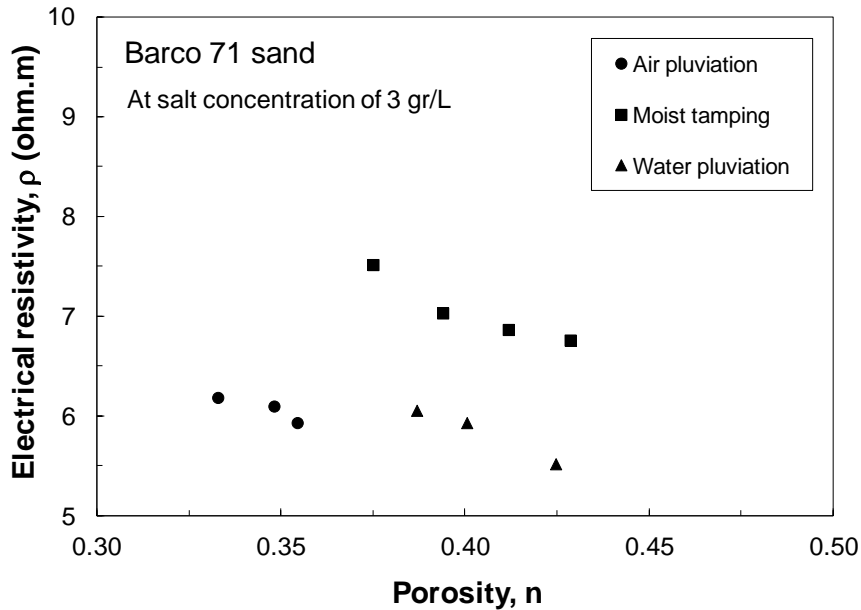


239
 240 Figure 6: Effect of sand particle size distribution and fines content (FC) on electrical resistivity
 241 with a pore water salt concentration of 3 gr/L
 242

243 *Effect of specimen preparation method*

244 In order to investigate the effect of sand fabric on the electrical resistivity of saturated sands, a
 245 number of experiments were conducted on Barco 71 sand samples prepared by air and water
 246 pluviation methods. These methods resemble sand fabrics formed by natural aeolian and fluvial
 247 process (e.g. in rivers and streams), respectively (Oda, et al., 1978, Vaid and Eliadorani, 1998).
 248 Air pluviated specimens were prepared by raining sand particles into the cylindrical mold
 249 through air. The porosity and density of the samples prepared by air pluviation was varied by
 250 changing the free-fall height and thus the depositional velocity of the particles. For preparing
 251 very loose samples, sand particles were rained with nearly zero drop height. A similar procedure
 252 was used to prepare water pluviated specimens by raining sand particles through water.
 253 However, as the particles reach a terminal velocity after a certain drop height in water, lower
 254 porosities were produced by tapping the sides of the specimen mold and thus densifying the sand
 255 sample. Figure 7 shows the results of these experiments. Electrical resistivity of the samples

256 prepared by the pluviation methods fall nearly on the same trendline, despite the larger porosities
 257 of the water pluviated samples. However, the electrical resistivities of the moist tamped samples
 258 are slightly greater which is likely because of the comparatively isolated pores of the honeycomb
 259 moist tamped sand fabric.
 260

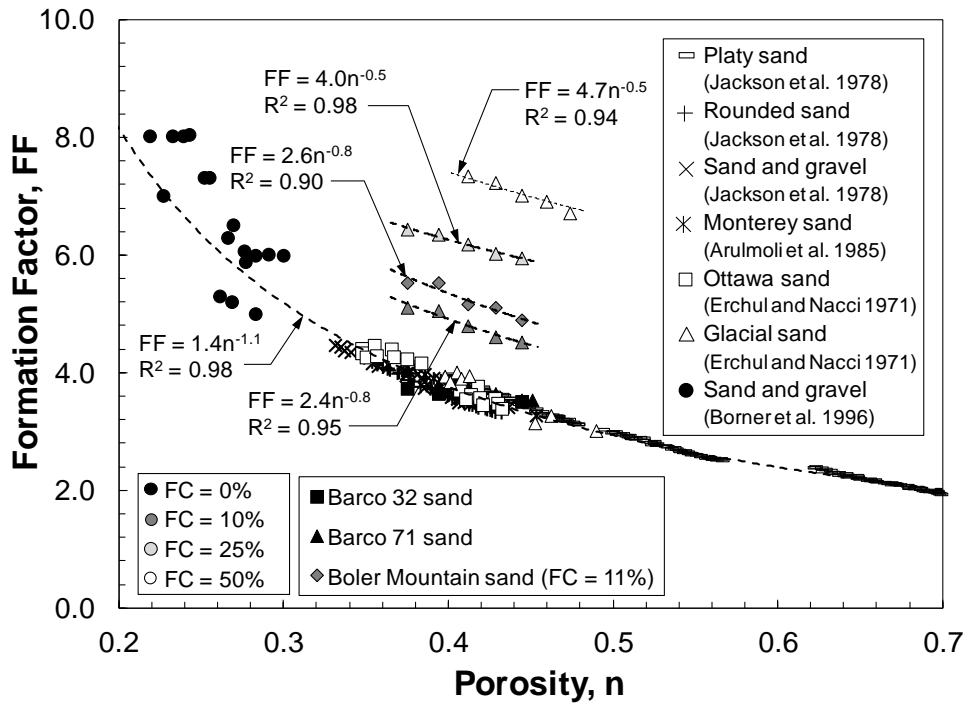


261
 262 Figure 7: Effect of specimen preparation method on electrical resistivity of Barco 71 sand
 263

264 *Estimation of porosity*

265 In order to develop a practical relationship, the effects of electrolyte salinity and probe geometry
 266 are removed by presenting the results in terms of formation factor (FF) and porosity (n) in Figure
 267 8. Electrical resistivity data for several other sands (platy sand, sand with 1% shell content,
 268 rounded sand, and sand and gravel) with different particle size distributions and particle shapes
 269 (Arulmoli, et al., 1985, Borner, et al., 1996, Jackson, et al., 1978) are also included in this figure.
 270 According to Figure 8, FF generally increases with decreasing sand porosity (or increasing
 271 density) due to the smaller water saturated pore volume of dense sands. According to these data,
 272 variations in gradation, mean particle size, and particle shape have little influence on the trend of
 273 data. The most significant changes in FF are produced by changes in FC, while the scatter at a
 274 certain FC are probably associated to variations in sand gradation and method of sample
 275 preparation.

276

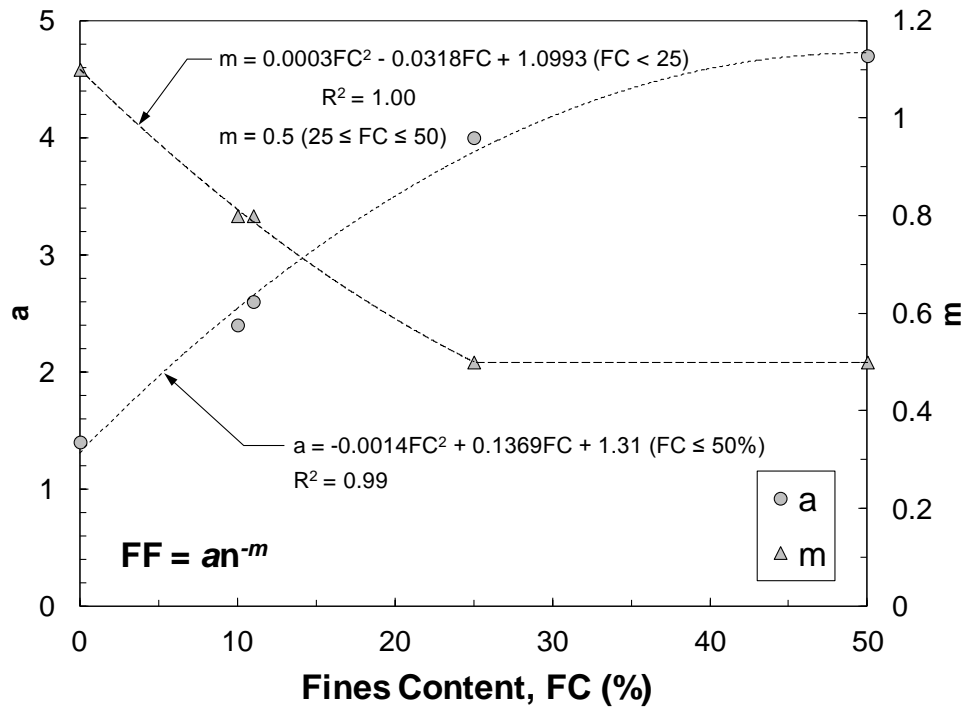


278

279 Figure 8: Effect of porosity on FF for the cohesionless soils tested in this study as well as for a
 280 platy sand, a rounded quartz sand, a quartz sand and gravel mixture (Jackson, et al., 1978)
 281 several other sands (Arulmoli, et al., 1985, Borner, et al., 1996, Jackson, et al., 1978)

282

283 Data at certain FC are also curve fitted with Equation [1] and the fitting parameters (a , m) are
 284 shown in Figure 8. In contrast to other studies, rather than particle shape (Jackson, et al., 1978)
 285 or cementation (Archie, 1942), Figure 8 indicates that the parameters of Equation [1] largely
 286 depend on FC. Fine particles increase the tortuosity and thus the length of the path that an
 287 electrical current must follow as well as constricting the openness of these flow channels. And
 288 therefore FF increases with increasing FC as a result of increasing tortuosity (Lesmes and
 289 Friedman, 2005, Schon, 2004). Figure 9 illustrates that coefficient a , and the exponent m
 290 respectively increase and decrease with increasing FC. For sands of this study, the specific
 291 relationships of these parameters with FC are also provided in Figure 9. As the experiments of
 292 this study were performed at small stress levels (< 5 kPa) the effect of stress level cannot be
 293 inferred from these results. However, based on past experimental observations (Hulbert, et al.,
 294 1982, Lee, et al., 2008, Lewis, et al., 1988), FF varies with stress to the extent that soil porosity
 295 is altered by stress. In other words, at a certain porosity stress does not appear to affect FF .



297

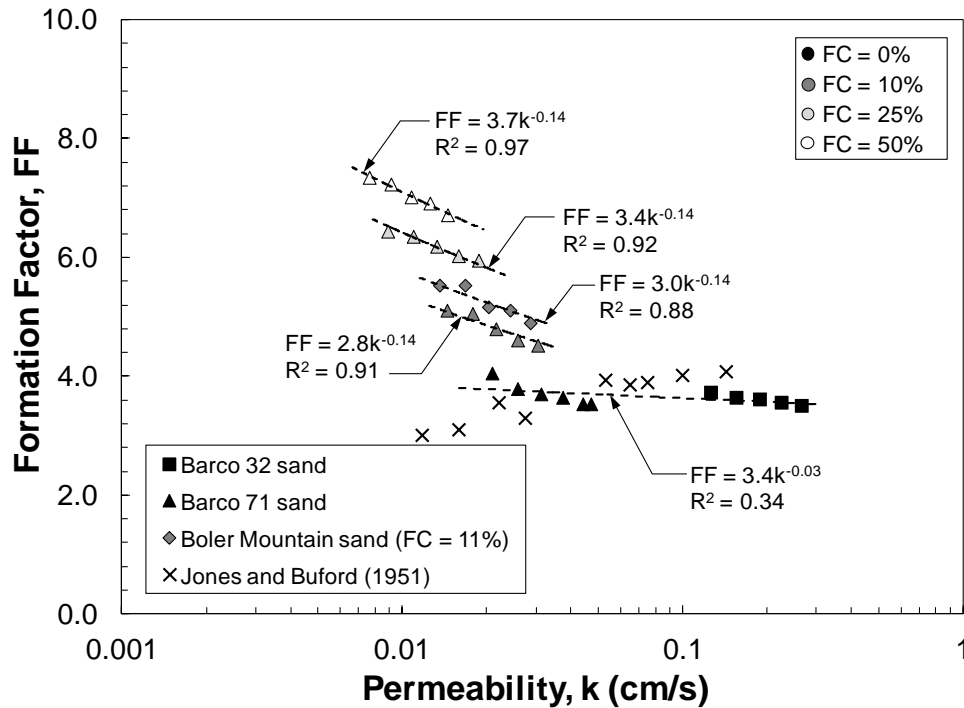
298 Figure 9: Variations of the parameters of Equation [1] with FC for the sands of this study

299

300 *Estimation of Hydraulic Conductivity*

301 Besides electrical resistivity, the coefficient of permeability (k) of the specimens was obtained by
 302 preparing saturated samples (at porosities similar to the electrical resistivity tests) in a constant-
 303 head permeameter cell. The permeability of these specimens was then measured following the
 304 ASTM D2434 (2006b) standard procedure for constant-head permeability tests. In this method,
 305 the sample is subjected to a water flow with a constant pressure head while the rate of flow and
 306 therefore k are measured. This procedure provides representative values of k that may occur in
 307 natural deposits or in embankments. Figure 10 shows unique relationships (similar to Eq. [1])
 308 between the results of these experiments with FF as well as those from Jones and Buford (1951)
 309 at certain FC. These data show that a higher FF would indicate an aquifer with higher
 310 permeability and yield. The good relationship between FF and permeability is somewhat
 311 expected as porosity and permeability are directly related and both electrical current and fluid
 312 flow move in tortuous paths through the intergranular pores of a soil. Particularly in sandy soils
 313 where there is no interaction between the ionic constituents of the porewater and the matrix solid,
 314 the bulk resistivity of a soil-water system would be a function of pore volume tortuosity, and

315 porosity and hence permeability. As illustrated in Figure 10, the addition of fine particles affects
 316 the pore sizes and the resulting permeability more significantly than porosity, as permeability is
 317 approximately proportional to the square of the pore sizes (Hazen, 1911). With increasing fines
 318 content, the porosity exponent increases from 0.03 to 0.14.
 319



320
 321 Figure 10: Variations of hydraulic conductivity and FF for the cohesionless soils of this study as
 322 well as for a sand from Jones and Buford (1951)
 323

324 A number of studies (Borner, et al., 1996, Pape, et al., 1987) have related hydraulic conductivity
 325 to the formation factor and the specific surface area of sediments (S_A). For example, based on a
 326 modified form of the Kozeny-Carman’s hydraulic conductivity relationship, Pape et al. (1987)
 327 suggested the following equation (known as the “PARIS” equation) for predicting hydraulic
 328 conductivity:

329

$$330 \quad k = \frac{\gamma_w}{\eta_w} \cdot \frac{475}{FFS_A^{3.1}} \quad \text{Equation [2]}$$

331
 332

333 In which, γ_w ($= 9.81 \text{ kN/m}^3$) and η_w ($=1.002 \times 10^{-6} \text{ kPa.s}$) are the unit weight and dynamic
334 viscosity of water at 20°C . Later, using spectral induced polarization as well as complex
335 electrical resistivity measurements, Borner et al. (1996) found the following proportionality from
336 their laboratory experiments:

337

$$338 \quad k \cong \frac{1}{\text{FFS}_A^c} \quad \text{Equation [3]}$$

339

340 in which S_A is determined from electrical measurements and the exponent c is in the range of 2.8
341 to 4.6. Similar to the Kozeny-Carman's equation, the application of Equations (2), and (3) is
342 limited because of the difficulty in determining S_A of sediments which normally requires
343 nitrogen gas adsorption measurements (Hillel, 1980). Although S_A was not measured for the
344 sediments of this study, correlations of Figure 10 suggest a modified form of Equation [3] as
345 below:

346

$$347 \quad k = \frac{A}{\text{FF}^c} \quad \text{Equation [4]}$$

348

349 In which, the coefficient A could include S_A of the corresponding sand. The difference with
350 Equations (2) and (3) results from the exponent ("c") applied to FF. Note that the plots of Figures
351 8 to 10 as well as the findings of this paper are only applicable to saturated cohesionless soils
352 with non-conductive particles. The result might be inapplicable for clayey, organic, or cemented
353 soils as the surface conduction of clay particles could dominate the electrical resistivity of a
354 clayey soil (Erickson and Jarrard, 1998).

355 Site specific calibration is recommended for using electrical resistivity measurements for
356 estimating sand porosity, yet in the absence of such data the correlations presented in Figures 8
357 and 9 can be employed to determine the in-situ density and seepage characteristics of sands.
358 Following the determination of in-situ porosity, laboratory sand specimens can be prepared at
359 representative porosities for the estimation of the in-situ strength of sandy soils.

360 Since changes in electrical resistivity result from changes in pore water volume, the application
361 of this method would require saturated soil conditions. Determination of the formation factor

362 also requires a separate measurement of the pore water resistivity. Offshore and surficial seabed
363 sediments would thus be the ideal conditions for the application of this method as the salinity of
364 the pore waters would vary little from that of the seawater (Siever, et al., 1965). The in-situ
365 resistivity of pore water (ρ_f) – which is needed to calculate FF - can be readily measured just
366 before the probe penetrates into the sediment without taking water samples. This can provide
367 relatively quick and inexpensive measurements of in-situ porosity and permeability of
368 cohesionless sediments.

369

370 **Conclusions**

371 The results of this study show that the electrical resistivity of cohesionless soils decreases with
372 increasing pore water salinity, and porosity while soil fabric, particle size distribution and shape
373 of the particles have negligible effects on electrical resistivity. However, the amount of silt
374 particles (between 0.002 to 0.075 mm, according to the Unified Soil Classification System) has a
375 profound impact on the resistivity of a saturated cohesionless soil to an electrical current.
376 Accordingly, a number of correlations are developed between electrical resistivity, and porosity
377 and hydraulic conductivity of cohesionless soils for certain fines contents. These results suggest
378 that electrical resistivity can provide a useful measurement for estimating the porosity and
379 permeability of cohesionless soils through Archie's law. Measurement of porosity and in-situ
380 density of cohesionless soils is essential for predicting their behavior under shear loading.
381 However this could be difficult due to sample disturbance and the inherent variability of in-situ
382 cohesionless soils. In the absence of site specific data, the relationships developed in this study
383 could be employed to determine the in-situ density and hydraulic conductivity of saturated
384 cohesionless soils indirectly from electrical resistivity measurements.

385

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